

ANEXO 4 – ESTUDO DE TRÁFEGO

EIV URE VALORIZA



URE VALORIZA ENERGIA SPE SANTOS/SP



ESTUDO DE TRÁFEGO (Outubro/ 2019)

URE Valoriza Energia SPE SANTOS/SP

ESTUDO DE TRÁFEGO

Relatório Final

(Outubro/ 2019)

SUMÁRIO

APRESENTAÇÃO

1. CARACTERIZAÇÃO

- 1.1. Localização
- 1.2. Características do Empreendimento
- 1.3. Operação Atual
 - 1.3.1. Transporte de Resíduos
 - 1.3.2. Número de Funcionários na Operação Atual
- 1.4. Fase de Implantação
 - 1.4.1. Transporte de Equipamentos e Materiais
 - 1.4.2. Número de Funcionários na Fase de Implantação
- 1.5. Operação Futura
 - 1.5.1. Coleta e Transporte de Resíduos
 - 1.5.2. Número de Funcionários na Fase de Operação do Empreendimento

2. FLUXO DE PASSAGEM

- 2.1. Fluxo de Passagem Existente
- 2.2. Fatores Aplicados aos Dados de Fluxo

3. COMPOSIÇÃO DOS CENÁRIOS DE ANÁLISE

4. ESTIMATIVA DE DEMANDA

- 4.1. Geração de Viagens
- 4.2. Divisão Modal
- 4.3. Distribuição Temporal

5. ANÁLISE DO IMPACTO

- 5.1. Segmentos Analisados
- 5.2. Resultados

6. CONCLUSÃO

- 6.1. Avaliação Final

7. ANEXOS

ÍNDICE

APRESENTAÇÃO

A TTC Engenharia de Tráfego e de Transportes Ltda. foi contratada pela SGW Services para a elaboração do Estudo de Tráfego integrante do Estudo de Impacto Ambiental (EIA) e Relatório de Impacto Ambiental (RIMA), para a Unidade de Recuperação de Energia (URE Valoriza Santos), a qual irá utilizar resíduos sólidos urbanos (RSU) resultante da coleta pública realizada em 7 municípios da Baixada Santista.

O empreendimento em questão possui as seguintes características:

- **Tipo de Uso:** Unidade de Recuperação de Energia - URE
- **Local:** km 254+900 da Rodovia Cônego Domênico Rangoni (SP-055)

O relatório foi elaborado com base nas informações fornecidas pelo empreendedor quanto ao cronograma implantação do empreendimento, dados atuais de entradas e saídas de caminhões transportadores de resíduos para o Aterro CGR Terrestre Ambiental, e dados futuros das entradas e saídas dos caminhões de resíduos da URE Valoriza Energia SPE, nos tipos de atividades a serem desenvolvidas no empreendimento. Baseou-se também em dados obtidos em contagens de fluxo realizadas na Rodovia Cônego Domênico Rangoni; além de reuniões e esclarecimentos realizados entre as equipes técnicas da TTC e da SGW Services.

O estudo do empreendimento teve como escopo básico:

- Levantamento das características físicas e operacionais do empreendimento e do sistema viário de acesso;
- Levantamento das distribuições modal, temporal e espacial, das viagens geradas pelo empreendimento;
- Identificação da contribuição do tráfego gerado pelo empreendimento na operação atual e futura, considerando o acréscimo de tráfego gerado pela implantação do referido empreendimento.

São Paulo, Outubro de 2019

TTC Engenharia de Tráfego e de Transportes Ltda.

1. CARACTERIZAÇÃO

1.1. Localização

A URE Valoriza Energia SPE será implantada no município de Santos, no Estado de São Paulo, no km 254,9 da pista leste, sentido Guarujá, da Rodovia Cônego Domênico Rangoni (SP-055), onde a velocidade regulamentada é de 90km/h. A localização do empreendimento é apresentada na Figura 1.1

Figura 1.1 – Localização



Fonte: SGW Services

1.2. Características do Empreendimento

O empreendimento em estudo é uma Unidade de Recuperação de Energia (URE) que utilizará resíduos sólidos urbanos provenientes da coleta pública realizada em 7 dos 9 municípios da Baixada Santista, os quais atualmente são encaminhados para o aterro CGR Terrestre.

Os resíduos sólidos urbanos, RSU, que serão tratados termicamente para geração de energia na URE Valoriza serão provenientes da coleta de resíduos realizada nos municípios de Santos, São Vicente, Guarujá, Cubatão, Mongaguá, Bertioga e Praia Grande pelas empresas Terracom (pertencente ao Grupo Terrestre Ambiental) e Transluc.

O acesso ao empreendimento será pelo acesso existente na Rodovia Cônego Domênico Rangoni, conforme apresentado na Figura 1.2.

Figura 1.2 – Acesso ao Empreendimento



Fonte: TTC

1.3. Operação Atual

1.3.1. Transporte de Resíduos

As empresas, responsáveis pela logística de coleta, transbordo e descarga do RSU no CGR Terrestre, contam com uma frota de 61 veículos conforme detalhado na planilha a seguir. O transporte desses resíduos será mantido, não havendo alterações após a implantação da URE Valoriza.

Tabela 1.1 – Frota Utilizada no Transporte de Resíduos

FROTA EMPREGADA PELA TERRACOM/ TRANSLUC PARA TRANSPORTE DE RESÍDUOS DOS MUNICÍPIOS PARA A TERRESTRE								
Município	Coletor 15m³	Coletor 19m³	Carreta (30t)	Agregados (basculantes)		Poliguindaste	Carreta (29t)	Observações
			V=55m³	Trucado	Toco	Caçamba V=4,8m³	V=25m³	
Bertioga			2					
Guarujá			7					
Santos			8					
Cubatão	2	8		5	6	2		
São Vicente	9	11						
Praia Grande							média 8vg/dia	Não é Terracom
Mongaguá			1					

Fonte: Terrestre Ambiental

Esta frota opera nos períodos diurno e noturno para atender a demanda de coleta e disposição de resíduos no aterro continuamente.

No quadro abaixo apresentamos dados de entrada de veículos no aterro CGR Terrestre durante os 3 primeiros meses do ano de 2019.

Tabela 1.2 – Dados de Entrada e Saída do Aterro: Atual

Mês	Total Viagens Entrada	Média Viagens Dia	Média Viagens Hora	Horário Trabalho	
				Início	Final
Janeiro	4.660	155	8	07:20	23:40
Fevereiro	3.650	130	6	07:20	23:40
Março	3.952	132	7	07:20	23:40

Fonte: Terrestre Ambiental

Segundo dados da Terrestre Ambiental, na hora mais carregada do dia 01 de julho de 2019, uma segunda-feira, dia da semana com maior fluxo no aterro, entraram no aterro 19 caminhões. A Tabela abaixo demonstra os municípios de origem e a quantidade de caminhões de cada um na hora mais carregada.

Tabela 1.3 – Viagens por Município na Hora Mais Carregada

MUNICÍPIO DE ORIGEM	ENTRADAS
BERTIOGA	1
CUBATÃO	9
SANTOS	2
GUARUJÁ	2
MONGAGUÁ	2
PRAIA GRANDE	3

Fonte: TTC com dados da Terrestre Ambiental

1.3.2. Número de Funcionários na Operação Atual

O CGR Terrestre conta com cerca de 92 colaboradores envolvidos na operação do aterro, divididos em dois turnos de 8 horas, conforme apresentado na Tabela 1.4.

Tabela 1.4 –Funcionários na Operação Atual

HORÁRIO DE TRABALHO ATUAL - ATERRO SANITÁRIO CGR TERRESTRE										
1	Mão de Obra	Segunda-feira a Sábado				Domingo				Nº de Funcionários
		Entrada	Saída	Entrada	Saída	Entrada	Saída	Entrada	Saída	
	Turno 01	07:20			15:40	07:20			15:40	92
	Turno 02	15:20			23:40					

Fonte: Terrestre Ambiental

A maioria dos funcionários acessa o CGR Terrestre utilizando o sistema de transporte fornecido pela empresa nos dois turnos: dois ônibus, que partem de Cubatão, e uma van, que parte de Santos na área das barcas de Vicente de Carvalho.

Apenas uma pequena parcela, cerca de cinco funcionários, utilizam veículo próprio para se deslocar até o local de trabalho.

1.4. Fase de Implantação

Durante a fase de implantação do empreendimento, o serviço de coleta de resíduos será mantido com o mesmo fluxo de caminhões.

1.4.1. Transporte de Equipamentos e Materiais

Todo o transporte de equipamentos e materiais necessários para a implantação da URE serão de responsabilidade da Valoriza Energia e serão realizados por empresas especializadas em movimentação de cargas dentro das normas vigentes.

Para as cargas especiais, devido ao peso, comprimento ou largura os transportes serão realizados em conformidade com o disposto no Código de Trânsito Brasileiro, com todas as providências de licenciamentos e pagamento de taxas necessárias para o tráfego.

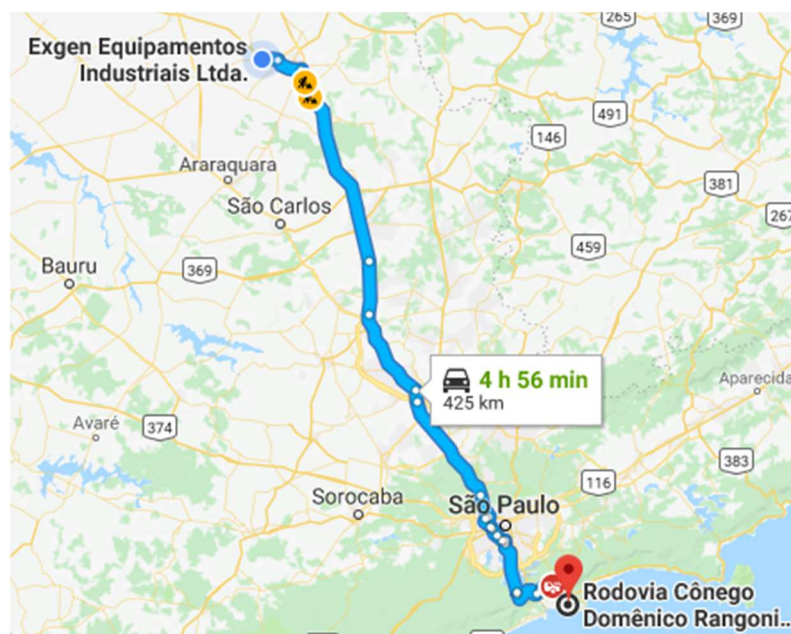
A previsão para a execução deste transporte é de 36 meses a partir da liberação das bases de fundações no canteiro de obras.

No planejamento da Valoriza Energia, todos os materiais, insumos e consumíveis enviados ao canteiro de obras serão recebidos, conferidos e armazenados em suas devidas áreas de acondicionamento, obedecendo procedimentos padrão para garantir a qualidade, a proteção e o controle de utilização e rastreabilidade.

Para a execução dos serviços de transportes, levou-se em consideração que todos os equipamentos, periféricos e acessórios pertencentes ao projeto serão expedidos a partir do Município de Sertãozinho, SP, onde se localiza a empresa de EPC (Engineering, Procurement e Construction) que irá executar as obras de implantação do empreendimento. A Exgen Equipamentos, empresa parceira da Ribeirão Energia, com parque industrial no Município de Sertãozinho, deverá produzir a caldeira e seus componentes, estruturas metálicas, *pipe racks* e acessórios em geral. A expedição dos equipamentos e componentes adquiridos na Exgen e fabricados por outros fornecedores serão expedidos diretamente das unidades fabris para o canteiro de obra.

Estes materiais serão transportados através das rodovias Carlos Tonani, Rodo Anel Viário Sul – Ribeirão Preto, Anhanguera, Bandeirantes, Rodo Anel Viário Sul – São Paulo, Imigrantes ou Anchieta e Conego Domenico Rangoni, local de implantação do projeto, conforme apresentado na Figura 1.3.

Figura 1.3 – Percurso do Transporte de Equipamentos entre Exgen e URE Valoriza



Fonte: Valoriza Energia

Na Tabela 1.5 são apresentados os totais de viagens de máquinas e equipamentos que serão utilizados durante a fase de implantação da URE.

Tabela 1.5 – Tráfego de Máquinas e Equipamentos na Fase de Implantação

EQUIPAMENTOS IMPLANTAÇÃO URE VALORIZA						
Equipamentos	Aplicação	Quantidade	Transporte	Rota	Periodicidade	Observação
Retroescavadeira	Construção	2	Prancha/Rodoviário	1	2 Viagens	Uma viagem na mobilização e outra na desmobilização
Caminhões Betoneiras – Concreto	Civil	360	Rodoviário	2	2 viagens/dia	-
Escavadeiras		1	Prancha/Rodoviário	1	2 Viagens	Uma viagem na mobilização e outra na desmobilização
Tratores		1	Prancha/Rodoviário	1	2 Viagens	
Trator Esteira D6		1	Prancha/Rodoviário	1	2 Viagens	
Caminhão Basculante		5	Rodoviário	1	2 Viagens	
Caminhão Pipa		1	Rodoviário	1	2 Viagens	
Perfuratrizes para Estacas		2	Carreta/Rodoviário	1	2 Viagens	
Rolo Compressor		1	Prancha/Rodoviário	1	2 Viagens	
Bombas de concreto		1	Carreta/Rodoviário	1	2 Viagens	
Bombas de água		2	Carreta/Rodoviário		2 Viagens	
Serra para corte de madeiras		2	Container/Rodoviário	1	2 Viagens	Uma viagem na mobilização e outra na desmobilização
Vibradores para concreto		5				
Ferramental para armadores		5				
Ferramental para Carpinteiros		5				
Ferramental para Pedreiros	5					

Equipamento para Topografia		2				
Betoneiras		5				
Guindaste Pequeno Porte – 25 Ton	Montagem Mecânica	2	Rodoviário	3	2 Viagens	Uma viagem na mobilização e outra na desmobilização
Guindaste Médio Porte – 50 Ton		1	Rodoviário	3	2 Viagens	
Guindaste Médio Porte – 80 Ton		1	Rodoviário	3	2 Viagens	
Guindaste Grande Porte – 110 Ton		1	Rodoviário	3	2 Viagens	
Guindaste Grande Porte – 250 Ton		1**	Rodoviário	4	8 Viagens	
Caminhão Munck 15 Ton		3	Rodoviário	3	2 Viagens	
Retificadoras de Solda 400 Amp		20	Container/Rodoviário	3	2 Viagens	
Retificadoras de Solda 600 Amp	3					
Máquinas de Corte – Plasma	2					
Conjunto Oxi Corte Completos – Maçarico	10					
Esmerilhadeira 7"	20					
Esmerilhadeira 4 1/2"	20					
Furadeira de Bancada	2					
Furadeira Manual	5					
Calandra para chapas	1					
Mandrilhadora	2					

Painéis Elétricos		10				
Ferramental para Caldeireiros / Montadores		20				
Ferramental para Soldadores		20				
Ferramental para Mecânicos		2				
Ferramental para Alinhamento e Nivelamento		1				
Andaimes Tubo Roll		300				
Talha 3,0 Ton		5				
Catraca 15 Ton		5				
Plataforma Elevatória		1	Prancha/Rodoviário	3	2 Viagens	Uma viagem na mobilização e outra na desmobilização
Aparelho de Ultrason		1	Rodoviário	3	2 Viagens	
Maquina Manual para Curvar Tubos	Elétrica e Automação	1	Container/Rodoviário	3	2 Viagens	Uma viagem na mobilização e outra na desmobilização
Rosqueadeira		1				
Ferramental para Eletricistas		10				
Ferramental para Instrumentistas		10				
Utilitário e Automóveis	Gerenciamento	3	Rodoviário	5	4 viagens	2 viagens por dia, 2 automóveis (ida pela manhã e retorno no final do dia)
Utilitário e Automóveis	Gerenciamento	2	Rodoviário	5A	4 viagens	2 viagens na semana (ida na segunda-feira e retorno na sexta-feira)

Fonte: Ribeirão Energia

As rotas citadas na tabela anterior são detalhadas na Tabela 1.6 a seguir.

Tabela 1.6 – Tráfego de Equipamentos

Tráfego Equipamentos Construção Civil + Montadoras da URE e Gerenciamento - 36 Meses													
Área	Destinos			Horário Circulação		Conduções		Rotas	Dias Úteis Trabalhados	Número de Viagens (Previsão)	Tipo		
	Quantidade	Santos	Sertãozinho	S. Bernardo	Entrada	Saída	Quant.				Tipo	Leve	Pesado
Equipamentos Construções Cíveis	11	11	0	0	7:30 as 10:30	10:30 as 16:00	11	Carretas	1	780	22	0	22
Betoneiras de Concreto	360	360	0	0	7:30 as 10:30	10:30 as 16:00	360	Caminhão Betoneira	2	780	720	0	720
Equipamentos Montagens	10	0	10	0	7:30 as 10:30	10:30 as 16:00	10	Carretas	3	780	20	0	20
Guindaste Pesado	8	0	0	8	6:30 as 7:30	15:00 as 17:00	8	Guindaste Pesado	4	780	8	0	8
Gerenciamento de Obra	312	0	312	0	8:00 as 13:00	16:00 as 21:00	2	Automóvel	5	156 Semanas	624	624	0

Fonte: Ribeirão Energia

1.4.2. Número de Funcionários na Fase de Implantação

Durante a fase de implantação da URE, o número de funcionários, incluído profissionais da construção civil, montagem mecânica, elétrica / automação e gerenciamento de obra poderá atingir 188, dependendo das etapas da obra, conforme apresentado na Tabela 1.7.

Tabela 1.7 –Funcionários Durante a Implantação da URE

Tráfego Funcionários Implantação da URE - 36 Meses														
Funcionários (Pico dos Trabalhos)		Transportes Funcionários			Horário Circulação		Conduções		Rotas	Dias Uteis Trabalha dos	Número de Viagens (Previsão)	Tipo		
Área	Quantidade	Santos	Guarujá	V. Carvalho	Entrada	Saída	Quant.	Tipo				Leve	Pesado	
Construções Cíveis	65	65	0	0	6:30 as 7:30	17:00 as 18:00	4	3 Ônibus + 1 Automóvel	2	780	6.240	1.560	4.680	
Montagem Mecânica	70	0	70	0	6:30 as 7:30	17:00 as 18:00	4	3 Ônibus + 1 Automóvel	5A	780	6.240	1.560	4.680	
Montagem Elétrica / Automação	45	0	45	0	6:30 as 7:30	17:00 as 18:00	3	2 Ônibus + 1 Automóvel	5A	780	4.680	1.560	3.120	
Gerenciament o de Obra	8	0	8	0	7:00 as 8:00	18:00 as 18:30	2	Automóvel	5A	780	3.120	3.120	0	

Fonte: Ribeirão Energia

Na Tabela 1.8 são detalhadas as rotas de acesso de equipamentos e funcionários ao empreendimento na fase de implantação, também ilustradas na Figura 1.4 à Figura 1.15.

Tabela 1.8 – Rotas de Acesso ao Empreendimento

ROTA 1 – Terrestre Ambiental/ URE			
Número de Viagens	22 Viagens		
Rotas	Veículos		
	Leves	Pesados	
Ponto de Saída - V1	Rod. Cônego Domênico Rangoni, 611		0
Ponto de Chegada - V1	Rod. Cônego Domênico Rangoni, Km 254,9		
Ponto de Saída - V2	Rod. Cônego Domênico Rangoni, Km 254,9		0
Ponto de Chegada - V2	Rod. Cônego Domênico Rangoni, 611		

ROTA 2 – Terracon/ URE			
Número de Viagens	6.240 Viagens		
Rotas	Veículos		
	Leves	Pesados	
Ponto de Saída - V1	Rod. Cônego Domênico Rangoni, Km 264	780	2.340
Ponto de Chegada - V1	Rod. Cônego Domênico Rangoni, Km 254,9		
Ponto de Saída - V2	Rod. Cônego Domênico Rangoni, Km 254,9	780	2.340
Ponto de Chegada - V2	Rod. Cônego Domênico Rangoni, Km 264		

ROTA 3 – Exgen/ URE			
Número de Viagens	20 Viagens		
Rotas	Veículos		
	Leves	Pesados	
Ponto de Saída - V1	Av. Marginal Antonio Aragão, 411	0	10
Ponto de Chegada - V1	Rod. Cônego Domênico Rangoni, Km 254,9		
Ponto de Saída - V2	Rod. Cônego Domênico Rangoni, Km 254,9	0	10
Ponto de Chegada - V2	Av. Marginal Antonio Aragão, 411		

ROTA 4 – São Bernardo do Campo/ URE			
Número de Viagens	8 Viagens		
Rotas	Veículos		
	Leves	Pesados	
Ponto de Saída - V1	Município de São Bernardo do Campo	0	4
Ponto de Chegada - V1	Rod. Cônego Domênico Rangoni, Km 254,9		
Ponto de Saída - V2	Rod. Cônego Domênico Rangoni, Km 254,9	0	4
Ponto de Chegada - V2	Município de São Bernardo do Campo		

ROTA 5 – Exgen/ URE			
Número de Viagens	624 Viagens		
Rotas	Veículos		
	Leves	Pesados	
Ponto de Saída - V1	Av. Marginal Antonio Aragão, 411	312	0
Ponto de Chegada - V1	Rod. Cônego Domênico Rangoni, Km 254,9		
Ponto de Saída - V2	Rod. Cônego Domênico Rangoni, Km 254,9	312	0
Ponto de Chegada - V2	Av. Marginal Antonio Aragão, 411		

ROTA 5 A – URE/ Guarujá			
Número de Viagens	14.040 Viagens		
Rotas	Veículos		
	Leves	Pesados	
Ponto de Saída – V3	Município do Guarujá	3.120	3.900
Ponto de Chegada – V3	Rod. Cônego Domênico Rangoni, Km 254,9		
Ponto de Saída – V4	Rod. Cônego Domênico Rangoni, Km 254,9	3.120	3.900
Ponto de Chegada – V4	Município do Guarujá		

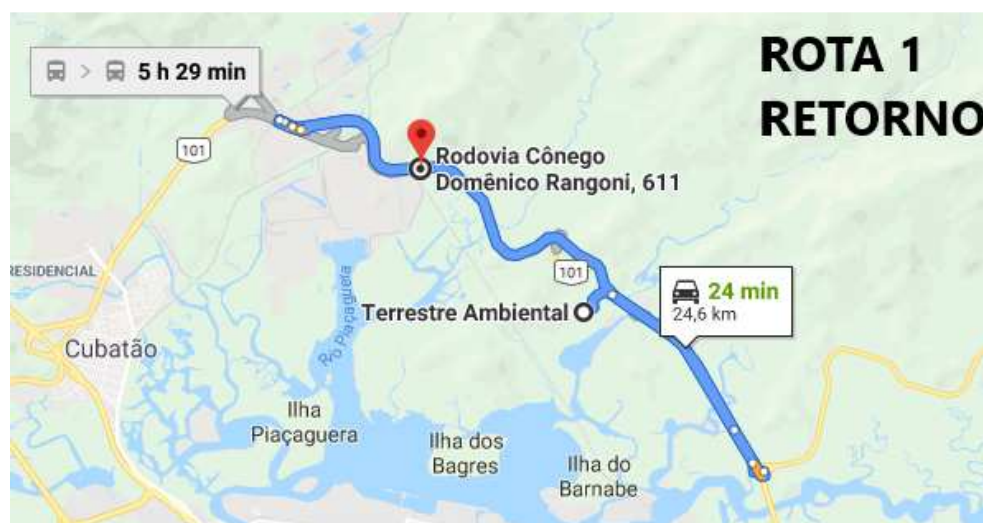
Fonte: Ribeirão Energia

Figura 1.4 – Rota 1: V1



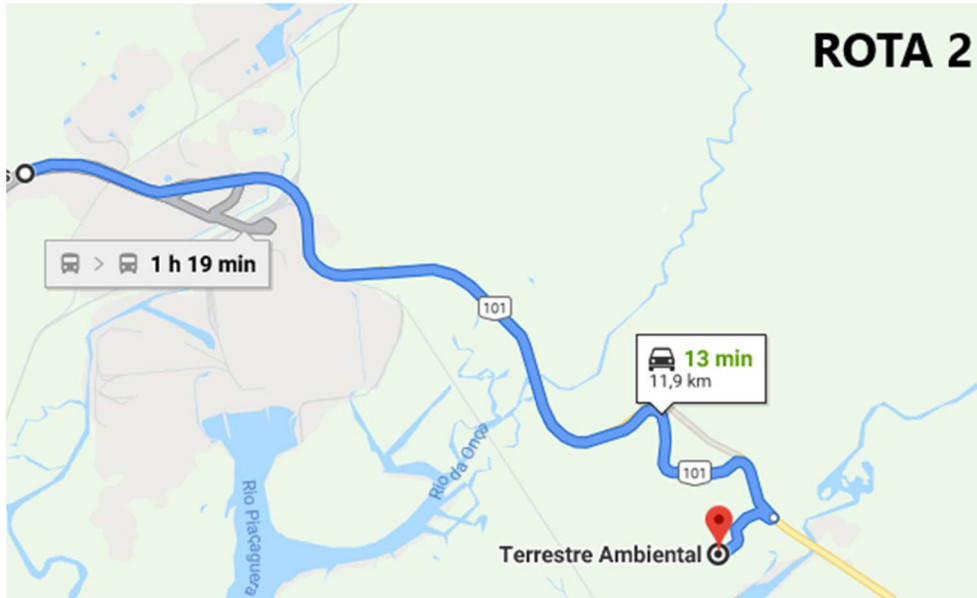
Fonte: Ribeirão Energia

Figura 1.5 – Rota 1: V2



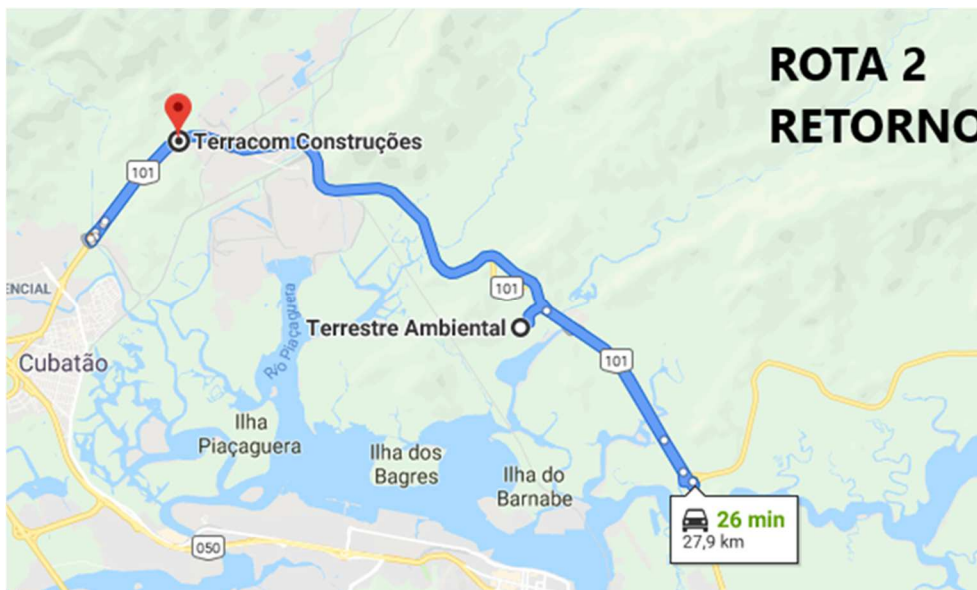
Fonte: Ribeirão Energia

Figura 1.6 – Rota 2: V1



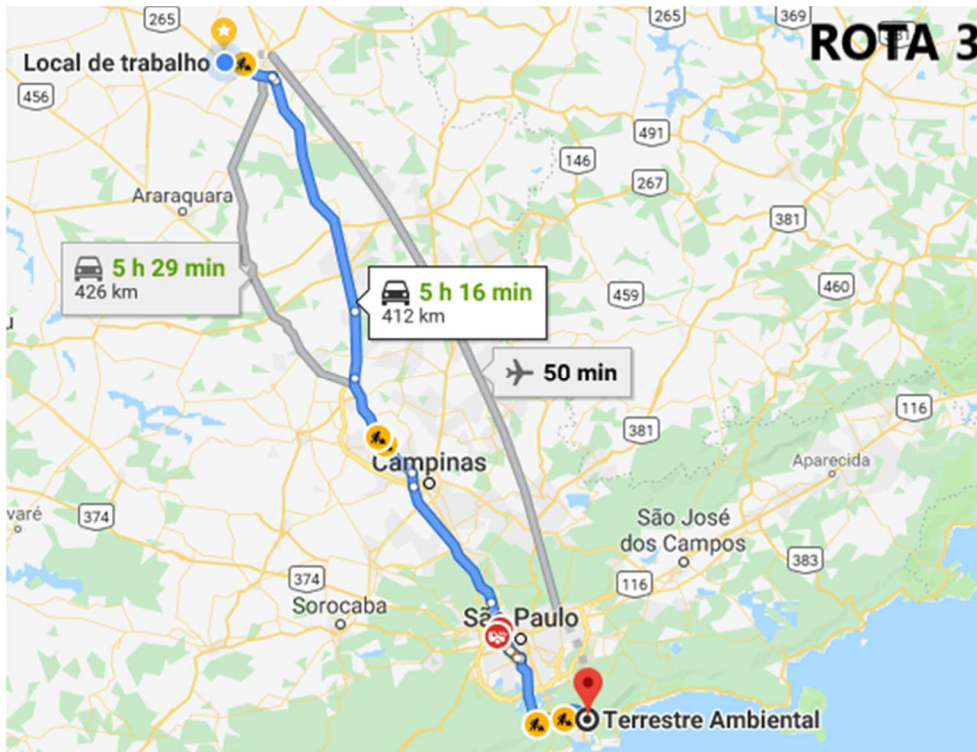
Fonte: Ribeirão Energia

Figura 1.7 – Rota 2: V2



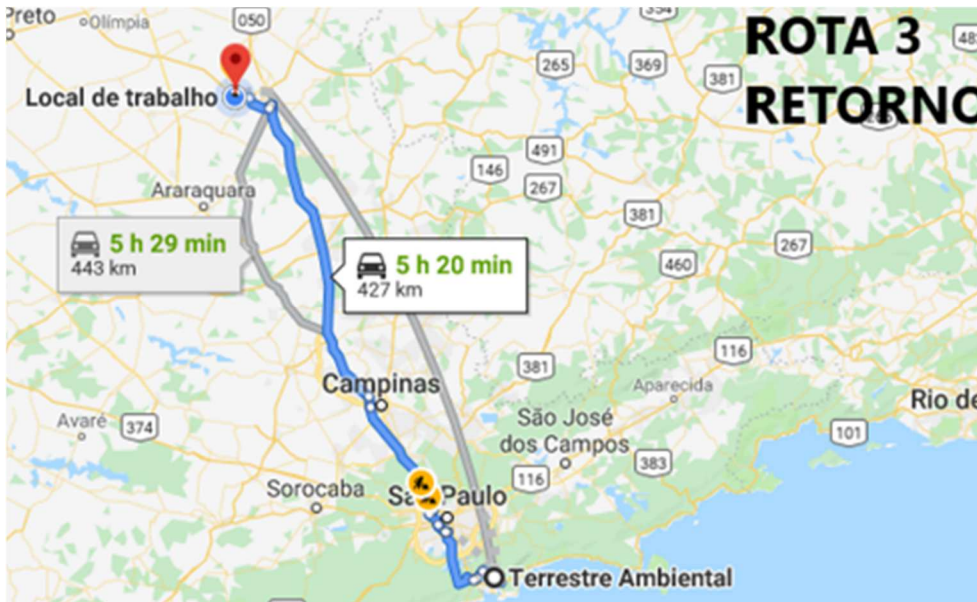
Fonte: Ribeirão Energia

Figura 1.8 – Rota 3: V1



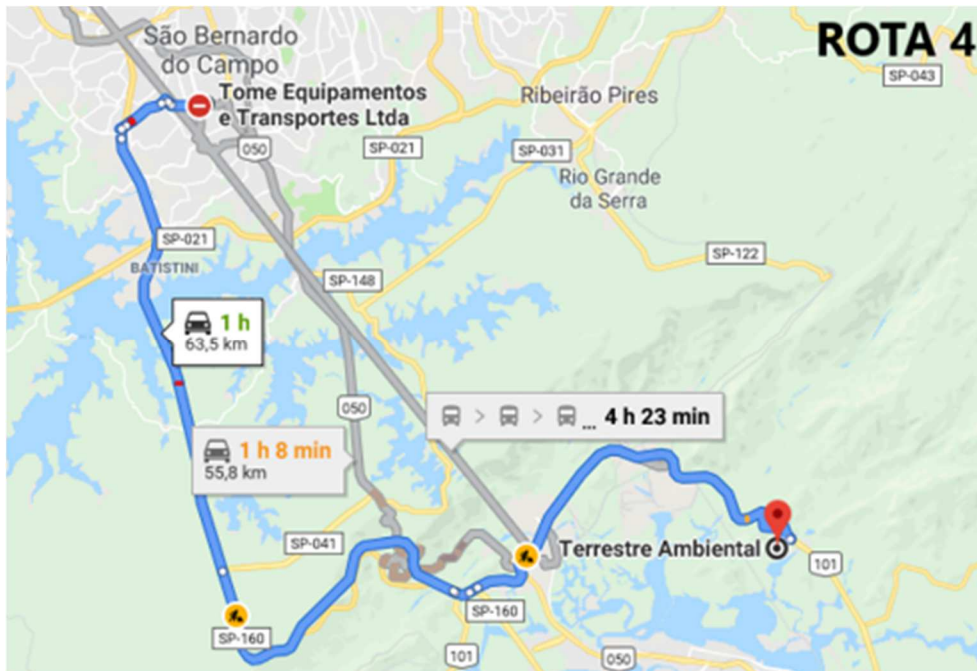
Fonte: Ribeirão Energia

Figura 1.9 – Rota 3: V2



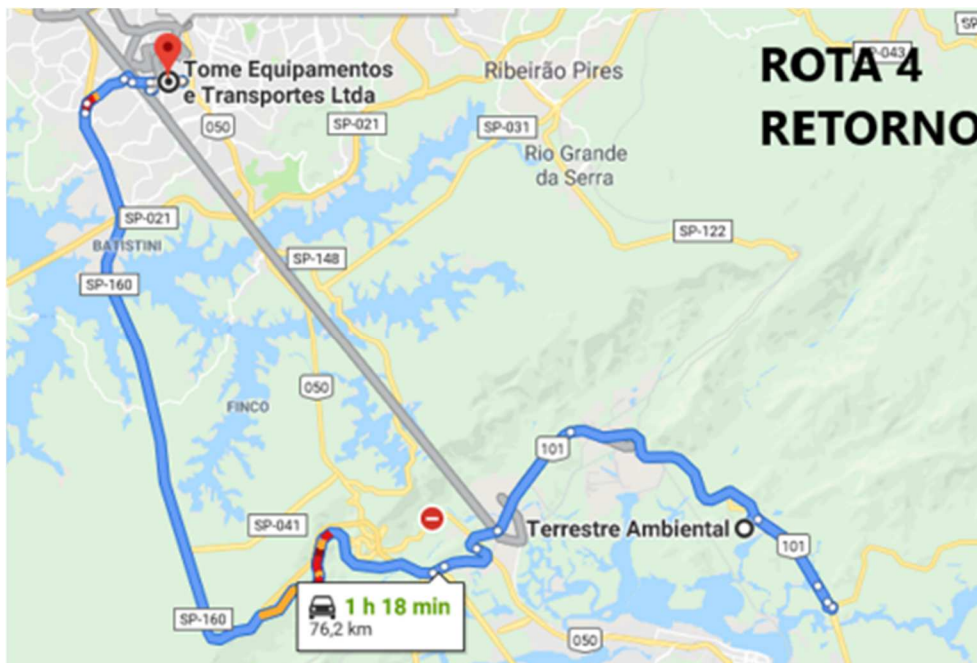
Fonte: Ribeirão Energia

Figura 1.10 – Rota 4: V1



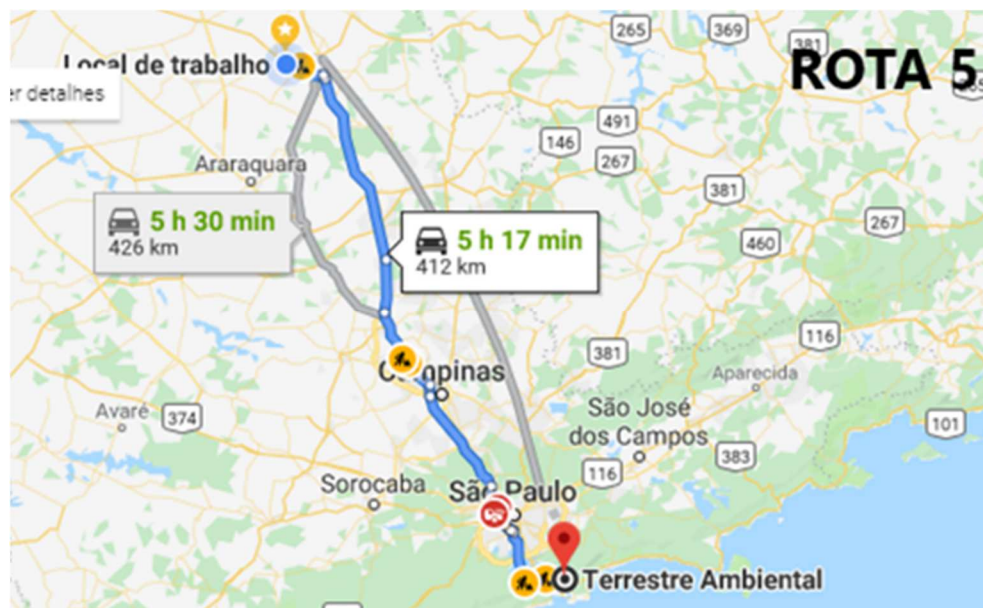
Fonte: Ribeirão Energia

Figura 1.11 – Rota 4: V2



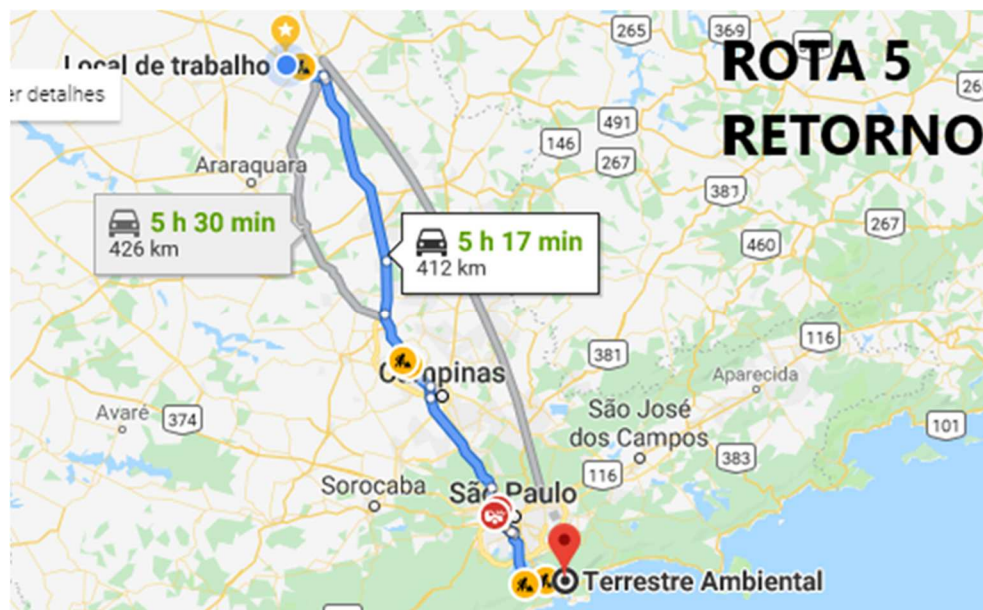
Fonte: Ribeirão Energia

Figura 1.12 – Rota 5: V1



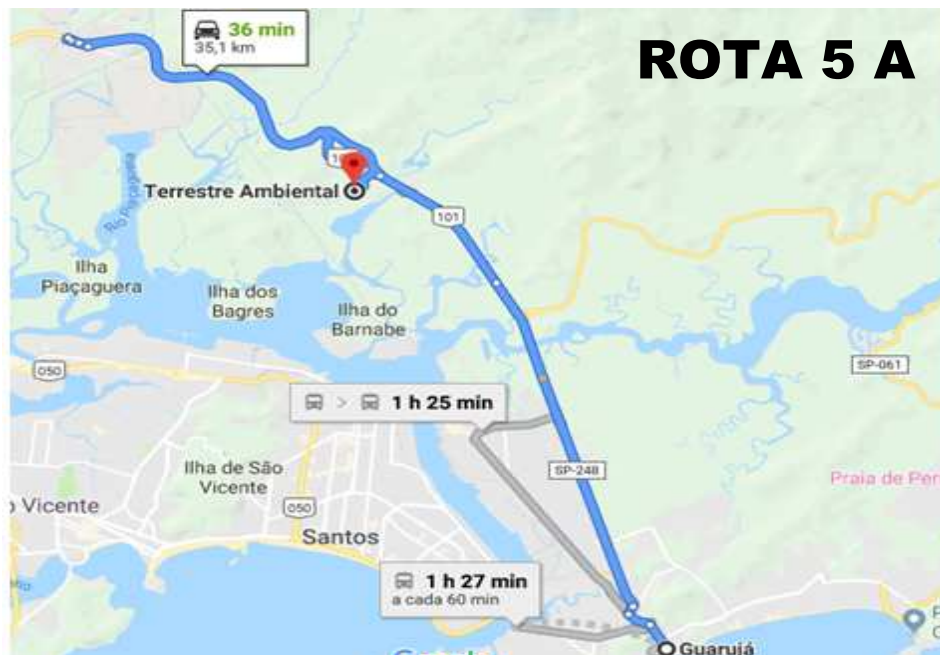
Fonte: Ribeirão Energia

Figura 1.13 – Rota 5: V2



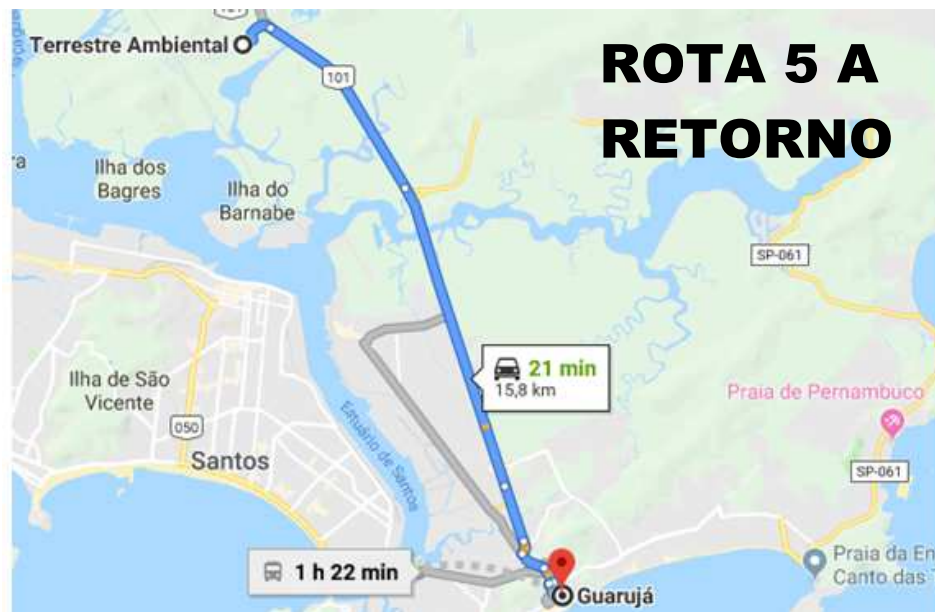
Fonte: Ribeirão Energia

Figura 1.14 – Rota 5 A: V3



Fonte: Ribeirão Energia

Figura 1.15 – Rota 5 A: V4



Fonte: Ribeirão Energia

1.5. Operação Futura

1.5.1. Coleta e Transporte de Resíduos

Na operação futura, será mantida a frota de veículos existentes, que continuarão coletando os resíduos dos municípios de Santos, Guarujá, Cubatão, Mongaguá, Bertioga e Praia Grande. Atualmente o aterro não recebe resíduos do município de São Vicente, mas futuramente poderá passar a receber resíduos deste município.

Nas figuras a seguir são apresentados os trajetos que serão percorridos pelos veículos para cada um dos municípios citados.

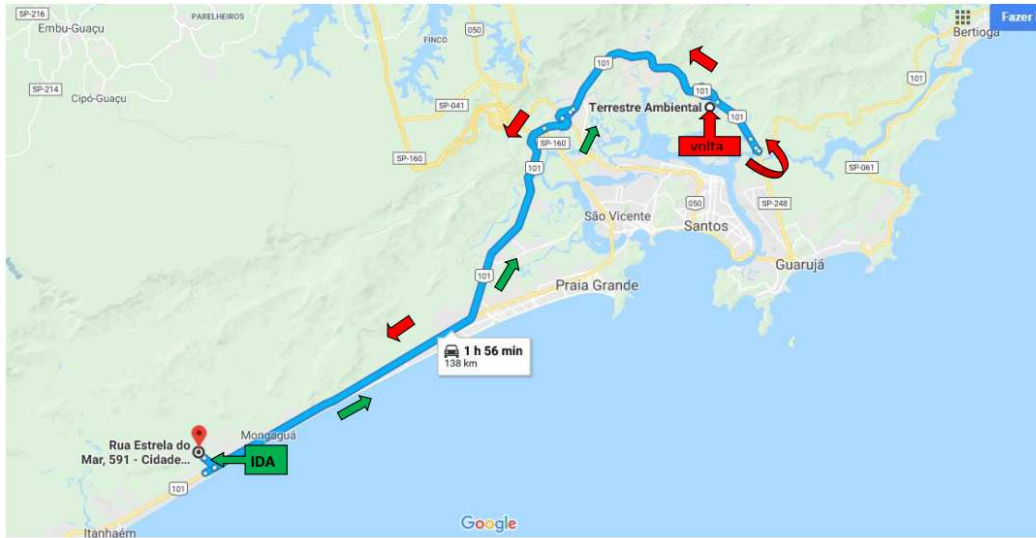
Figura 1.16 – Trajeto do Transbordo: Praia Grande



Endereço: Avenida do Trabalhador, 2.300 – Jardim Guaramar - Praia Grande/SP.

Fonte: Ribeirão Energia

Figura 1.17 – Trajeto do Transbordo: Mongaguá



Endereço: Rua Estrela do Mar, 591-649 - Cidade Balneário Marinho - Mongaguá/SP.

Fonte: Ribeirão Energia

Figura 1.18 – Trajeto São Vicente

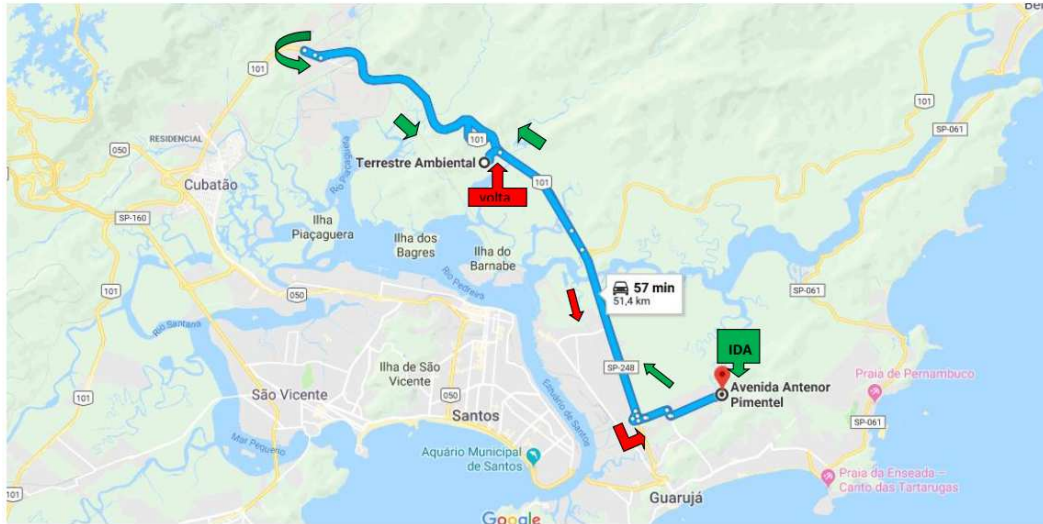


O Transbordo de São Vicente está desativado, estamos utilizando o endereço da garagem da Terracom.

Endereço: Rua Frei Gaspar, 3092 – Parque São Vicente – São Vicente/SP.

Fonte: Ribeirão Energia

Figura 1.19 – Trajeto do Transbordo: Guarujá



Endereço: Avenida Antenor Pimentel, s/n – Morrinhos – Guarujá/SP.

Fonte: Ribeirão Energia

Figura 1.20 – Trajeto do Transbordo: Santos



Endereço: Rua C, s/n – Alemoa – Santos/SP.

Fonte: Ribeirão Energia

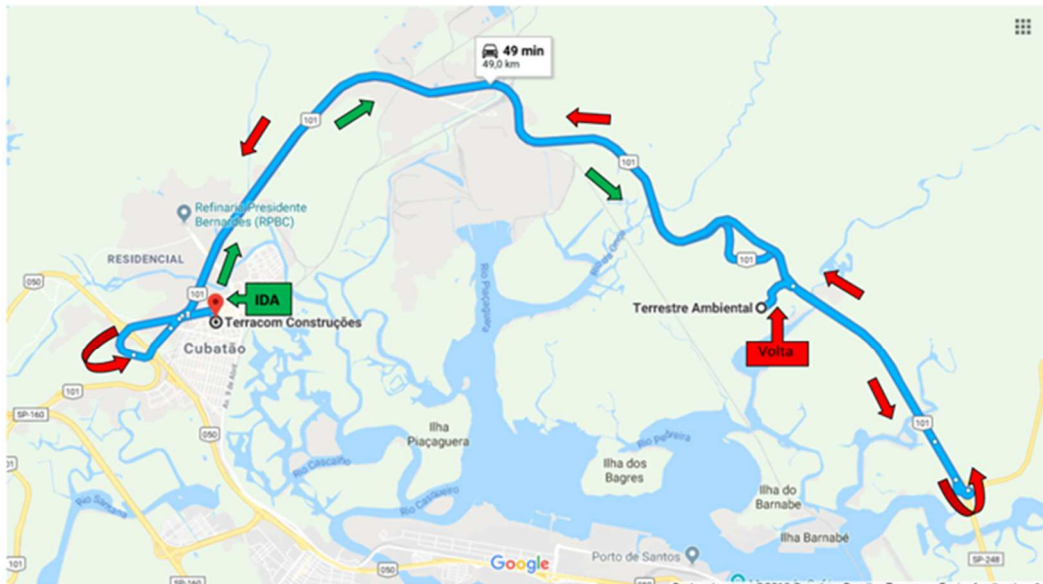
Figura 1.21 – Trajeto do Transbordo: Bertioga



Endereço: Rodovia Manoel Hipólito Rego Km 227 – Bertioga/SP.

Fonte: Ribeirão Energia

Figura 1.22 – Trajeto Cubatão



Não há Transbordo de resíduos no município, estamos utilizando o endereço da garagem da Terracom.

Endereço: Rua Professora Ana Dias, 162 - Vila Paulista, Cubatão/SP.

Fonte: Ribeirão Energia

1.5.2. Número de Funcionários na Fase de Operação do Empreendimento

Na Tabela 1.9 apresenta-se o número de funcionários na Fase de Operação da URE, que deverá operar em três turnos de 8 horas cada.

Tabela 1.9 –Funcionários na Fase de Operação Futura da URE

HORÁRIO DE TRABALHO - OPERAÇÃO URE VALORIZA										
Mão de Obra	Segunda-feira a Sábado				Domingo				Nº de Funcionários	
	Entrada	Saída	Entrada	Saída	Entrada	Saída	Entrada	Saída		
3	Turno 01	07:20			15:40	07:20			15:40	88
	Turno 02	15:20			23:40	15:20			23:40	
	Turno 03	23:20			07:20	23:20			07:20	
	Comercial	07:20			16:20	NA			NA	

Fonte: Ribeirão Energia

Observa-se que o número de funcionários é muito próximo do atual. Assim, estima-se que serão mantidas as viagens do transporte de passageiros existente.

Na Tabela 1.10 é apresentado o quadro detalhado de funcionários da URE.

Tabela 1.10 – Quadro de Funcionários da URE

QUADRO DE FUNCIONÁRIOS	Nível	Regime Mão de Obra	Regime Trabalho	Número de Funcionários por Turno	Número Total Funcionários		Detalhamento Funções
						88	
Gerência Geral - Coordenação Operacional / Manutenção							
1	Gerente/ Coordenador	Superior	MI - 1	Comercial	1	1	Gerenciamento Operacional da URE Gerenciamento Plano de Manutenção da URE Gerenciamento Plano de Fornecimento Combustíveis e Insumos Gerenciamento Plano de Controle Ambiental Gerenciamento Indicadores de Performance e Custos da URE
Operação URE - Utilidades							
2	Líder de Turno Geração de Vapor e Energia Elétrica	Técnico	MD	Escala	1	3	Supervisiona atividades relacionadas a operação dos equipamentos, responsável pela disciplina e elabora escalas de trabalhos dos colaboradores. Auxilia na geração do plano de manutenção da URE.
3	Operador de Sala - Sistema Transporte de RSU, Bioestabilizadores e Transporte de CDR	Técnico	MD	Escala	1	3	Acompanha o funcionamento dos equipamentos via Supervisório e realiza ajustes quando necessário via computador ou solicita ao operador de campo para ajustes <i>in loco</i> .
4	Operador de Sala - ETA/ ETA Desmi, Caldeira e seus Periféricos + Sistema de Tratamento dos Gases	Técnico	MD	Escala	1	3	Acompanha o funcionamento dos equipamentos via Supervisório e realiza ajustes quando necessário via computador ou solicita ao operador de campo para ajustes <i>in loco</i> .
5	Operador de Sala- Turbogenerador, Torres de Resfriamento e Exportação de Energia	Técnico	MD	Escala	1	3	Acompanha o funcionamento dos equipamentos via Supervisório e realiza ajustes quando necessário via computador ou solicita ao operador de campo para ajustes <i>in loco</i> .
6		Técnico	MD	Escala	1	3	Acompanha o funcionamento dos equipamentos (Bombas/ Ventiladores/ Válvulas

	Operador ETA/ ETA Desmi e Torre de Resfriamento						Acompanha a qualidade de água e realiza ajustes conforme parâmetros exigidos
7	Operador de Campo - Caldeira/ Tratamento de Gás	Técnico	MD	Escala	1	3	Acompanha o funcionamento dos equipamentos e periféricos da caldeira, sendo roscas de cinzas, bombas d'água, grelhas de queima, desaerador, etc.
8	Operador de Campo - Turbina e Condensador	Técnico	MD	Escala	1	3	Acompanha o funcionamento dos equipamentos e periféricos da turbina e condensador, instrumentos, válvulas, bombas e manobras como troca de filtros de água e óleo, etc.
9	Operador de Pá Carregadeira	Médio	MD	Escala	1	3	Efetua a mistura e alimentação de combustível dentro do silo de armazenamento
10	Operador de Ponte Rolante e Garra Hidráulica	Médio	MD	Escala	2	6	Opera Ponte Rolante e Garra Hidráulica na alimentação do sistema de transporte de RSU - Alimentação do Abre Saco
11	Operador campo - recebimento RSU/ Sistema Transportes/ Tratamento	Médio	MD	Escala	2	8	Opera e acompanha o funcionamento dos equipamentos responsáveis pela mistura e alimentação de combustível (RSU) na caldeira.
12	Ajudante Prático	Médio	MD	Escala	1	3	Auxilia processo operacional da planta Executa limpezas nas áreas operacionais da planta - Caldeira, Turbo, Gerador, etc.
13	Operador de campo - resíduos gerados	Médio	MD	Escala	1	3	Responsável pela retirada transporte e armazenagem dos resíduos.
Apoio Operacional							
14	Técnico em Laboratório	Técnico	MD	Escala	1	3	Responsável pela coleta e análise de água, condensados, CDR e resíduos. Responsável pela qualidade de fluidos e resíduos e realiza ajustes conforme parâmetros exigidos por normas e fabricantes de equipamentos e insumos. Emite laudos laboratoriais.
15	Técnico Meio Ambiente	Técnico	MI	Comercial	1	1	Responsável pelo gerenciamento ambiental da URE seguindo as normas e demais legislações aplicáveis à operação. Gerenciamento dos programas e planos ambientais, bem como do monitoramento das águas subterrâneas e superficiais.
16	Assistente Administrativo	Técnico	MI - 1	Comercial	1	1	Auxilia e apoia o processo administrativo da planta térmica. Atende as necessidades burocráticas para compras, RH, etc.
17	Almoxarife	Técnico	MI - 1	Comercial	1	1	Responsável pelo recebimento, armazenagem, controle de estoque e distribuição de insumos, consumíveis, peças de reposição, etc.
Manutenção							
18	Supervisor Mecânica, Elétrica e Instrumentação	Superior	MI	Comercial	1	1	Coordena o Plano de Manutenção da Planta Térmica Responsável pela distribuição de tarefas das equipes de manutenção Elabora e acompanha controles e cronogramas de manutenções preventivas e corretivas.
19	Mecânico	Técnico	MI	Escala	1	3	Executa manutenção em equipamentos mecânicos, verificando causas de defeitos e/ou substituindo peças danificadas.
20	Caldereiro/ Montador	Técnico	MI	Comercial	1	1	Executa manutenção em equipamentos danificados pelo uso. Fabrica e monta novos equipamentos e dispositivos. Manutenção em Geral.
21	Eletricista NR10	Técnico	MI	Escala	1	3	Executa manutenção em equipamentos elétricos verificando causa de defeitos e/ou substituindo peças danificadas pelo uso. Manutenção preventiva e corretiva. Manutenção em alta e baixa tensão.
22	Soldador Tig/ Eletrodo Revestido	Técnico	MI	Comercial	1	1	Executa soldagens qualificadas com eletrodos, arames Mig e varetas Tig em máquinas, tubulações de alta e baixa pressão, estruturas industriais e equipamentos diversos.
23	Ajudante Prático	Médio	MI	Escala	4	12	Auxiliar na execução de atividades do departamento de manutenção.

24	Instrumentista	Técnico	MI	Escala	1	3	Executa manutenção em equipamentos e instrumentos de automação, verificando causas de defeitos e/ ou substituindo peças danificadas. Regulagem de instrumentos conforme parâmetros operacionais.
Administrativo/ Serviços Gerais							
25	Técnico Segurança Trabalho	Técnico	MI - 1	Comercial	1	1	Responsável pela segurança dos trabalhadores da URE, aplicando as normas trabalhistas (NRs do Ministério do Trabalho), incluindo uso de EPI's, segurança no uso dos equipamentos da URE, controle dos dispositivos de segurança na execução de tarefas, ergonomia elaboração de PPRA e PCMSO, etc.
26	Supervisor de Recursos Humanos	Superior	MI	Comercial	1	1	Recrutamento e seleção (triagem de currículos, entrevistas, testes). Treinamento e desenvolvimento da equipe. Cargos e salários (estrutura e descrição de cargos e funções, pesquisa salarial, política de remuneração, promoção e reajustes). Remuneração e benefícios. Folha de pagamento (admissão, controle de ponto, cálculos de folha, férias, rescisões) Relações trabalhistas (negociação com sindicato, processos trabalhistas).
27	Portaria Central/ Balanceiro	Médio	MI - 1	Escala	1	4	Responsável pelo controle de entrada e saída de funcionários e visitantes na URE. Responsável pelo controle de entrada e pesagem dos veículos de transporte de RSU, insumos e consumíveis.
28	Vigilante Segurança Patrimonial	Médio	MI	Escala	1	4	Responsável pela vigilância patrimonial da URE.
29	Auxiliar de Limpeza	Médio	MI	Comercial	2	2	Executa tarefas de limpeza e organização das áreas da URE.
30	Motorista	Médio	MI	Comercial	1	1	Apoio aos departamentos da URE.

Fonte: Ribeirão Energia

2. FLUXO DE PASSAGEM

2.1. Fluxo de Passagem Existente

Foram realizadas contagens veiculares classificadas na seção da rodovia e em movimentos específicos pelas rotas a serem utilizadas pelo empreendimento em um dia típico, das 06h – 22h, classificados em auto, ônibus, caminhão e moto realizada nos dias 25 e 27 de junho do 2019.

Nas figuras a seguir são apresentados os locais e movimentos contados e no ANEXO 1 podem ser consultadas as tabelas com a tabulação completa das contagens.

Figura 2.1 – Localização dos pontos de contagem



Fonte: TTC

Figura 2.2 – Movimentos de Contagem: Ponto 1



Fonte: TTC

Figura 2.3 – Movimentos de Contagem: Ponto 2



Fonte: TTC

Figura 2.4 – Movimentos de Contagem: Ponto 3



Fonte: TTC

Na Tabela 2.1, Tabela 2.2, Tabela 2.3 e Tabela 2.4 são apresentados os valores horários das contagens e no capítulo 7 ANEXOS são apresentados os dados coletados de 15 em 15 minutos.

Tabela 2.1 – Resumo das contagens: Movimentos 1 a 3 – 2019

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 1				MOVIMENTO 2				MOVIMENTO 3			
	Auto	Ônibus	Cam.	Moto	Auto	Ônibus	Cam.	Moto	Auto	Ônibus	Cam.	Moto
06:00 - 07:00	243	16	181	185	16	0	101	2	57	0	23	13
07:00 - 08:00	305	10	152	115	5	0	72	2	90	1	24	7
08:00 - 09:00	257	3	223	45	6	0	106	1	116	0	19	9
09:00 - 10:00	260	4	232	39	10	0	128	0	121	1	18	9
10:00 - 11:00	219	6	167	41	10	0	128	0	91	0	27	7
11:00 - 12:00	238	2	230	23	15	3	131	0	93	1	10	7
12:00 - 13:00	165	2	190	18	9	0	97	0	82	1	9	6
13:00 - 14:00	255	4	186	48	9	0	103	1	72	1	19	5
14:00 - 15:00	235	6	262	77	13	0	135	0	58	1	19	7
15:00 - 16:00	308	2	181	46	10	0	107	2	65	1	12	14
16:00 - 17:00	328	6	243	34	19	0	125	1	68	0	7	6
17:00 - 18:00	355	7	206	82	18	1	152	3	74	0	8	8
18:00 - 19:00	287	4	181	61	11	1	53	2	69	1	7	11
19:00 - 20:00	165	8	157	35	7	0	43	0	49	0	6	8
20:00 - 21:00	93	3	124	15	4	0	46	1	24	2	0	0
21:00 - 22:00	90	3	94	14	1	0	39	0	21	0	1	1
Total	3803	86	3009	878	163	5	1566	15	1150	10	209	118

Elaboração: Cambury – Adaptação TTC

Cam.: Caminhões

Tabela 2.2 – Resumo das contagens: Movimentos 4 a 6 – 2019

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 4				MOVIMENTO 5				MOVIMENTO 6			
	Auto	Ônibus	Cam.	Moto	Auto	Ônibus	Cam.	Moto	Auto	Ônibus	Cam.	Moto
06:00 - 07:00	5	0	0	0	43	1	5	9	139	5	244	54
07:00 - 08:00	3	0	0	0	66	1	6	10	363	7	316	85
08:00 - 09:00	3	0	2	0	83	1	9	2	281	7	226	13
09:00 - 10:00	3	0	1	0	96	1	12	5	304	10	242	28
10:00 - 11:00	1	0	0	0	111	0	20	8	283	3	229	21
11:00 - 12:00	4	0	0	0	94	0	21	6	235	6	230	24
12:00 - 13:00	4	0	0	0	93	1	20	4	253	2	228	32
13:00 - 14:00	1	0	0	0	102	0	21	14	253	7	195	24
14:00 - 15:00	3	0	1	1	117	1	23	6	183	8	260	20
15:00 - 16:00	2	0	0	1	144	2	26	12	174	5	237	43
16:00 - 17:00	4	0	0	1	149	1	32	11	198	7	266	32
17:00 - 18:00	4	0	0	1	142	1	20	12	312	9	222	92
18:00 - 19:00	5	0	0	1	111	1	15	2	225	9	227	77
19:00 - 20:00	3	0	0	0	69	1	9	6	238	13	192	39
20:00 - 21:00	5	0	0	0	32	0	5	1	150	7	159	16
21:00 - 22:00	0	0	0	2	23	1	3	6	128	6	154	14
Total	50	0	4	7	1475	13	247	114	3719	111	3627	614

Elaboração: Cambury – Adaptação TTC

Cam.: Caminhões

Tabela 2.3 – Resumo das contagens: Movimentos 7 a 9 – 2019

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 7 - ENTRADA NO ATERRO				MOVIMENTO 8 - SAÍDA DO ATERRO				MOVIMENTO 9			
	Auto	Ônibus	Cam.	Moto	Auto	Ônibus	Cam.	Moto	Auto	Ônibus	Cam.	Moto
06:00 - 07:00	0	0	0	0	0	0	0	0	10	4	9	9
07:00 - 08:00	5	2	14	0	2	0	7	0	18	18	21	14
08:00 - 09:00	1	0	13	0	2	0	13	0	16	33	23	0
09:00 - 10:00	2	0	11	0	1	0	19	0	20	4	30	2
10:00 - 11:00	2	0	18	0	3	0	13	0	21	1	29	4
11:00 - 12:00	4	0	18	0	3	0	20	0	9	3	32	1
12:00 - 13:00	1	0	15	0	2	0	14	0	11	0	38	0
13:00 - 14:00	4	0	14	0	0	0	14	0	15	4	28	4
14:00 - 15:00	1	0	11	0	4	0	9	0	15	5	45	5
15:00 - 16:00	0	0	15	0	0	1	11	0	14	6	30	2
16:00 - 17:00	2	0	10	0	2	0	17	0	15	2	31	6
17:00 - 18:00	2	0	2	0	5	1	4	0	14	0	39	3
18:00 - 19:00	1	0	4	0	0	0	3	0	6	2	32	2
19:00 - 20:00	1	0	2	0	0	0	4	0	4	2	22	9
20:00 - 21:00	0	0	2	0	0	0	4	0	1	0	22	4
21:00 - 22:00	0	0	0	0	0	0	0	0	5	1	18	3
Total	26	2	149	0	24	2	152	0	194	85	449	68

Elaboração: Cambury – Adaptação TTC

Cam.: Caminhões

Tabela 2.4 – Resumo das contagens: Movimentos 10 e 11 – 2019

VOLUME POR MOVIMENTO								
Hora	MOVIMENTO 10				MOVIMENTO 11			
	Auto	Ônibus	Cam.	Moto	Auto	Ônibus	Cam.	Moto
06:00 - 07:00	15	11	17	3	105	12	9	28
07:00 - 08:00	30	13	16	7	187	26	12	75
08:00 - 09:00	28	17	11	6	46	13	15	9
09:00 - 10:00	26	16	14	5	39	13	22	4
10:00 - 11:00	35	10	19	3	38	10	16	5
11:00 - 12:00	36	13	27	7	27	10	13	6
12:00 - 13:00	31	12	22	5	42	8	15	15
13:00 - 14:00	27	13	22	6	23	12	12	8
14:00 - 15:00	29	7	17	8	34	9	13	12
15:00 - 16:00	54	16	18	7	38	12	16	6
16:00 - 17:00	91	13	22	29	29	10	11	4
17:00 - 18:00	139	28	19	52	28	13	16	3
18:00 - 19:00	50	14	21	12	29	11	18	9
19:00 - 20:00	32	10	19	13	20	10	17	4
20:00 - 21:00	10	9	14	1	15	9	17	1
21:00 - 22:00	12	10	11	7	11	10	7	3
Total	645	212	289	171	711	188	229	192

Elaboração: Cambury – Adaptação TTC

Cam.: Caminhões

A partir dos movimentos de contagem, estimou-se o volume na seção da rodovia no sentido de Guarujá (pista leste) e Cubatão (pista oeste), conforme apresentado na Tabela 2.5.

Tabela 2.5 – Volume na Rodovia SP-055 – 2019

VOLUME POR MOVIMENTO									
HORÁRIO	PISTA OESTE - CUBATÃO				PISTA LESTE - GUARUJÁ				TOTAL
	Auto	Ônibus	Caminhão	VEIC. EQ.	Auto	Ônibus	Caminhão	VEIC. EQ.	VEIC. EQ.
06:00 - 07:00	297	17	287	753	212	5	368	772	1.525
07:00 - 08:00	373	11	230	735	458	8	412	1.088	1.823
08:00 - 09:00	343	4	336	853	403	7	351	940	1.793
09:00 - 10:00	363	5	371	927	435	11	388	1.034	1.961
10:00 - 11:00	339	6	315	821	384	3	384	965	1.785
11:00 - 12:00	343	5	382	924	343	10	371	915	1.838
12:00 - 13:00	263	3	307	728	344	3	334	850	1.578
13:00 - 14:00	365	4	310	836	334	8	317	822	1.658
14:00 - 15:00	362	7	419	1.001	254	9	414	889	1.890
15:00 - 16:00	460	4	314	937	249	6	356	792	1.729
16:00 - 17:00	492	7	400	1.103	285	7	398	893	1.995
17:00 - 18:00	511	9	378	1.092	404	10	382	992	2.084
18:00 - 19:00	404	6	249	787	305	11	287	752	1.539
19:00 - 20:00	238	9	209	565	294	13	241	675	1.240
20:00 - 21:00	124	3	175	391	178	9	205	499	890
21:00 - 22:00	114	4	136	324	150	6	194	450	774
Total	5.391	104	4.818	12.774	5.032	126	5.402	13.324	26.098

Elaboração: TTC

Observa-se que a hora com maior volume na Rodovia, em veículos equivalentes, é entre 17h00 e 18h00, que será a hora analisada.

2.2. Fatores Aplicados aos Dados de Fluxo

O fator de crescimento do fluxo, com base nas informações dos últimos dez anos fornecidas pelo Índice **ABCR, da Associação Brasileira de Concessionárias de Rodovias**, disponíveis no portal <http://www.abcr.org.br/Conteudo/Secao/23/sao+paulo.aspx> foi de 2,3% ao ano. Por meio desse fator, os valores atuais do fluxo de passagem são extrapolados para o cenário futuro referente ao ano de 2020, ano de início de implantação da URE, 2023, ano da conclusão da implantação da URE, e 2033, dez anos após a conclusão da implantação do empreendimento.

Mediante a aplicação do fator de crescimento do fluxo de passagem sobre os valores dos movimentos de contagem obtidos em 2019 demonstrados nas Tabelas anteriores, obtêm-se os valores das Tabelas seguintes.

Tabela 2.6 – Resumo das contagens: Movimentos 1 a 3 – 2020

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 1				MOVIMENTO 2				MOVIMENTO 3			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	249	16	185	189	16	0	103	2	58	0	24	13
07:00 - 08:00	312	10	156	118	5	0	74	2	92	1	25	7
08:00 - 09:00	263	3	228	46	6	0	108	1	119	0	19	9
09:00 - 10:00	266	4	237	40	10	0	131	0	124	1	18	9
10:00 - 11:00	224	6	171	42	10	0	131	0	93	0	28	7
11:00 - 12:00	243	2	235	24	15	3	134	0	95	1	10	7
12:00 - 13:00	169	2	194	18	9	0	99	0	84	1	9	6
13:00 - 14:00	261	4	190	49	9	0	105	1	74	1	19	5
14:00 - 15:00	240	6	268	79	13	0	138	0	59	1	19	7
15:00 - 16:00	315	2	185	47	10	0	109	2	66	1	12	14
16:00 - 17:00	336	6	249	35	19	0	128	1	70	0	7	6
17:00 - 18:00	363	7	211	84	18	1	156	3	76	0	8	8
18:00 - 19:00	294	4	185	62	11	1	54	2	71	1	7	11
19:00 - 20:00	169	8	161	36	7	0	44	0	50	0	6	8
20:00 - 21:00	95	3	127	15	4	0	47	1	25	2	0	0
21:00 - 22:00	92	3	96	14	1	0	40	0	21	0	1	1
Total	3.891	88	3.078	898	167	5	1.602	15	1.176	10	214	121

Elaboração: TTC

Tabela 2.7 – Resumo das contagens: Movimentos 4 a 6 – 2020

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 4				MOVIMENTO 5				MOVIMENTO 6			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	5	0	0	0	44	1	5	9	142	5	250	55
07:00 - 08:00	3	0	0	0	68	1	6	10	371	7	323	87
08:00 - 09:00	3	0	2	0	85	1	9	2	287	7	231	13
09:00 - 10:00	3	0	1	0	98	1	12	5	311	10	248	29
10:00 - 11:00	1	0	0	0	114	0	20	8	290	3	234	21
11:00 - 12:00	4	0	0	0	96	0	21	6	240	6	235	25
12:00 - 13:00	4	0	0	0	95	1	20	4	259	2	233	33
13:00 - 14:00	1	0	0	0	104	0	21	14	259	7	199	25
14:00 - 15:00	3	0	1	1	120	1	24	6	187	8	266	20
15:00 - 16:00	2	0	0	1	147	2	27	12	178	5	242	44
16:00 - 17:00	4	0	0	1	152	1	33	11	203	7	272	33
17:00 - 18:00	4	0	0	1	145	1	20	12	319	9	227	94
18:00 - 19:00	5	0	0	1	114	1	15	2	230	9	232	79
19:00 - 20:00	3	0	0	0	71	1	9	6	243	13	196	40
20:00 - 21:00	5	0	0	0	33	0	5	1	153	7	163	16
21:00 - 22:00	0	0	0	2	24	1	3	6	131	6	158	14
Total	51	0	4	7	1.509	13	253	117	3.805	114	3.711	628

Elaboração: TTC

Tabela 2.8 – Resumo das contagens: Movimentos 7 a 9 – 2020

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 7 - ENTRADA NO ATERRO				MOVIMENTO 8 - SAÍDA DO ATERRO				MOVIMENTO 9			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	0	0	0	0	0	0	0	0	10	4	9	9
07:00 - 08:00	5	2	14	0	2	0	7	0	18	18	21	14
08:00 - 09:00	1	0	13	0	2	0	13	0	16	34	24	0
09:00 - 10:00	2	0	11	0	1	0	19	0	20	4	31	2
10:00 - 11:00	2	0	18	0	3	0	13	0	21	1	30	4
11:00 - 12:00	4	0	18	0	3	0	20	0	9	3	33	1
12:00 - 13:00	1	0	15	0	2	0	14	0	11	0	39	0
13:00 - 14:00	4	0	14	0	0	0	14	0	15	4	29	4
14:00 - 15:00	1	0	11	0	4	0	9	0	15	5	46	5
15:00 - 16:00	0	0	15	0	0	1	11	0	14	6	31	2
16:00 - 17:00	2	0	10	0	2	0	17	0	15	2	32	6
17:00 - 18:00	2	0	2	0	5	1	4	0	14	0	40	3
18:00 - 19:00	1	0	4	0	0	0	3	0	6	2	33	2
19:00 - 20:00	1	0	2	0	0	0	4	0	4	2	23	9
20:00 - 21:00	0	0	2	0	0	0	4	0	1	0	23	4
21:00 - 22:00	0	0	0	0	0	0	0	0	5	1	18	3
Total	27	2	152	0	25	2	156	0	198	87	459	70

Elaboração: TTC

Tabela 2.9 – Resumo das contagens: Movimentos 10 e 11 – 2020

VOLUME POR MOVIMENTO								
Hora	MOVIMENTO 10				MOVIMENTO 11			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	15	11	17	3	107	12	9	29
07:00 - 08:00	31	13	16	7	191	27	12	77
08:00 - 09:00	29	17	11	6	47	13	15	9
09:00 - 10:00	27	16	14	5	40	13	23	4
10:00 - 11:00	36	10	19	3	39	10	16	5
11:00 - 12:00	37	13	28	7	28	10	13	6
12:00 - 13:00	32	12	23	5	43	8	15	15
13:00 - 14:00	28	13	23	6	24	12	12	8
14:00 - 15:00	30	7	17	8	35	9	13	12
15:00 - 16:00	55	16	18	7	39	12	16	6
16:00 - 17:00	93	13	23	30	30	10	11	4
17:00 - 18:00	142	29	19	53	29	13	16	3
18:00 - 19:00	51	14	21	12	30	11	18	9
19:00 - 20:00	33	10	19	13	20	10	17	4
20:00 - 21:00	10	9	14	1	15	9	17	1
21:00 - 22:00	12	10	11	7	11	10	7	3
Total	660	217	296	175	727	192	234	196

Elaboração: TTC

Tabela 2.10 – Volume na Rodovia SP-055 - 2020

VOLUME POR MOVIMENTO									
Hora	PISTA OESTE - CUBATÃO				PISTA LESTE - GUARUJÁ				TOTAL
	Auto	Ônibus	Caminhão	VEIC. EQ.	Auto	Ônibus	Caminhão	VEIC. EQ.	VEIC. EQ.
06:00 - 07:00	304	17	294	926	217	5	376	980	1.906
07:00 - 08:00	382	11	235	875	469	8	421	1.328	2.203
08:00 - 09:00	351	4	344	1.047	412	7	359	1.145	2.191
09:00 - 10:00	371	5	380	1.141	445	11	397	1.261	2.402
10:00 - 11:00	347	6	322	1.004	393	3	393	1.185	2.188
11:00 - 12:00	351	5	391	1.143	351	10	380	1.130	2.273
12:00 - 13:00	269	3	314	903	352	3	342	1.041	1.945
13:00 - 14:00	373	4	317	1.016	342	8	324	1.007	2.023
14:00 - 15:00	370	7	429	1.242	260	9	424	1.125	2.367
15:00 - 16:00	471	4	321	1.121	255	6	364	995	2.117
16:00 - 17:00	503	7	409	1.336	292	7	407	1.120	2.456
17:00 - 18:00	523	9	387	1.315	413	10	391	1.215	2.530
18:00 - 19:00	413	6	255	935	312	11	294	922	1.857
19:00 - 20:00	243	9	214	690	301	13	247	820	1.510
20:00 - 21:00	127	3	179	491	182	9	210	620	1.111
21:00 - 22:00	117	4	139	403	153	6	198	563	966
Total	5.515	106	4.929	15.586	5.148	129	5.526	16.459	32.045

Elaboração: TTC

Tabela 2.11 – Resumo das contagens: Movimentos 1 a 3 – 2023

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 1				MOVIMENTO 2				MOVIMENTO 3			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	266	18	198	203	18	0	111	2	62	0	25	14
07:00 - 08:00	334	11	167	126	5	0	79	2	99	1	26	8
08:00 - 09:00	282	3	244	49	7	0	116	1	127	0	21	10
09:00 - 10:00	285	4	254	43	11	0	140	0	133	1	20	10
10:00 - 11:00	240	7	183	45	11	0	140	0	100	0	30	8
11:00 - 12:00	261	2	252	25	16	3	143	0	102	1	11	8
12:00 - 13:00	181	2	208	20	10	0	106	0	90	1	10	7
13:00 - 14:00	279	4	204	53	10	0	113	1	79	1	21	5
14:00 - 15:00	257	7	287	84	14	0	148	0	64	1	21	8
15:00 - 16:00	337	2	198	50	11	0	117	2	71	1	13	15
16:00 - 17:00	359	7	266	37	21	0	137	1	74	0	8	7
17:00 - 18:00	389	8	226	90	20	1	167	3	81	0	9	9
18:00 - 19:00	314	4	198	67	12	1	58	2	76	1	8	12
19:00 - 20:00	181	9	172	38	8	0	47	0	54	0	7	9
20:00 - 21:00	102	3	136	16	4	0	50	1	26	2	0	0
21:00 - 22:00	99	3	103	15	1	0	43	0	23	0	1	1
Total	4.166	94	3.296	962	179	5	1.715	16	1.260	11	229	129

Elaboração: TTC

Tabela 2.12 – Resumo das contagens: Movimentos 4 a 6 - 2023

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 4				MOVIMENTO 5				MOVIMENTO 6			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	5	0	0	0	47	1	5	10	152	5	267	59
07:00 - 08:00	3	0	0	0	72	1	7	11	398	8	346	93
08:00 - 09:00	3	0	2	0	91	1	10	2	308	8	248	14
09:00 - 10:00	3	0	1	0	105	1	13	5	333	11	265	31
10:00 - 11:00	1	0	0	0	122	0	22	9	310	3	251	23
11:00 - 12:00	4	0	0	0	103	0	23	7	257	7	252	26
12:00 - 13:00	4	0	0	0	102	1	22	4	277	2	250	35
13:00 - 14:00	1	0	0	0	112	0	23	15	277	8	214	26
14:00 - 15:00	3	0	1	1	128	1	25	7	200	9	285	22
15:00 - 16:00	2	0	0	1	158	2	28	13	191	5	260	47
16:00 - 17:00	4	0	0	1	163	1	35	12	217	8	291	35
17:00 - 18:00	4	0	0	1	156	1	22	13	342	10	243	101
18:00 - 19:00	5	0	0	1	122	1	16	2	246	10	249	84
19:00 - 20:00	3	0	0	0	76	1	10	7	261	14	210	43
20:00 - 21:00	5	0	0	0	35	0	5	1	164	8	174	18
21:00 - 22:00	0	0	0	2	25	1	3	7	140	7	169	15
Total	55	0	4	8	1.616	14	271	125	4.074	122	3.973	673

Elaboração: TTC

Tabela 2.13 – Resumo das contagens: Movimentos 7 a 9 - 2023

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 7 - ENTRADA NO ATERRO				MOVIMENTO 8 - SAÍDA DO ATERRO				MOVIMENTO 9			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	0	0	0	0	0	0	0	0	11	4	10	10
07:00 - 08:00	5	2	15	0	2	0	8	0	20	20	23	15
08:00 - 09:00	1	0	14	0	2	0	14	0	18	36	25	0
09:00 - 10:00	2	0	12	0	1	0	21	0	22	4	33	2
10:00 - 11:00	2	0	20	0	3	0	14	0	23	1	32	4
11:00 - 12:00	4	0	20	0	3	0	22	0	10	3	35	1
12:00 - 13:00	1	0	16	0	2	0	15	0	12	0	42	0
13:00 - 14:00	4	0	15	0	0	0	15	0	16	4	31	4
14:00 - 15:00	1	0	12	0	4	0	10	0	16	5	49	5
15:00 - 16:00	0	0	16	0	0	1	12	0	15	7	33	2
16:00 - 17:00	2	0	11	0	2	0	19	0	16	2	34	7
17:00 - 18:00	2	0	2	0	5	1	4	0	15	0	43	3
18:00 - 19:00	1	0	4	0	0	0	3	0	7	2	35	2
19:00 - 20:00	1	0	2	0	0	0	4	0	4	2	24	10
20:00 - 21:00	0	0	2	0	0	0	4	0	1	0	24	4
21:00 - 22:00	0	0	0	0	0	0	0	0	5	1	20	3
Total	28	2	163	0	26	2	167	0	213	93	492	74

Elaboração: TTC

Tabela 2.14 – Resumo das contagens: Movimentos 10 e 11 - 2023

VOLUME POR MOVIMENTO								
Hora	MOVIMENTO 10				MOVIMENTO 11			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	16	12	19	3	115	13	10	31
07:00 - 08:00	33	14	18	8	205	28	13	82
08:00 - 09:00	31	19	12	7	50	14	16	10
09:00 - 10:00	28	18	15	5	43	14	24	4
10:00 - 11:00	38	11	21	3	42	11	18	5
11:00 - 12:00	39	14	30	8	30	11	14	7
12:00 - 13:00	34	13	24	5	46	9	16	16
13:00 - 14:00	30	14	24	7	25	13	13	9
14:00 - 15:00	32	8	19	9	37	10	14	13
15:00 - 16:00	59	18	20	8	42	13	18	7
16:00 - 17:00	100	14	24	32	32	11	12	4
17:00 - 18:00	152	31	21	57	31	14	18	3
18:00 - 19:00	55	15	23	13	32	12	20	10
19:00 - 20:00	35	11	21	14	22	11	19	4
20:00 - 21:00	11	10	15	1	16	10	19	1
21:00 - 22:00	13	11	12	8	12	11	8	3
Total	707	232	317	187	779	206	251	210

Elaboração: TTC

Tabela 2.15 – Volume na Rodovia SP-055 - 2023

VOLUME POR MOVIMENTO									
Hora	PISTA OESTE - CUBATÃO				PISTA LESTE - GUARUJÁ				TOTAL
	Auto	Ônibus	Caminhão	VEIC. EQ.	Auto	Ônibus	Caminhão	VEIC. EQ.	VEIC. EQ.
06:00 - 07:00	325	19	314	991	232	5	403	1.049	2.041
07:00 - 08:00	409	12	252	937	502	9	451	1.422	2.358
08:00 - 09:00	376	4	368	1.121	441	8	384	1.226	2.346
09:00 - 10:00	398	5	406	1.221	477	12	425	1.351	2.572
10:00 - 11:00	371	7	345	1.075	421	3	421	1.268	2.343
11:00 - 12:00	376	5	418	1.224	376	11	406	1.210	2.434
12:00 - 13:00	288	3	336	967	377	3	366	1.115	2.082
13:00 - 14:00	400	4	340	1.088	366	9	347	1.078	2.166
14:00 - 15:00	397	8	459	1.330	278	10	453	1.205	2.535
15:00 - 16:00	504	4	344	1.201	273	7	390	1.066	2.266
16:00 - 17:00	539	8	438	1.431	312	8	436	1.199	2.630
17:00 - 18:00	560	10	414	1.408	443	11	418	1.301	2.709
18:00 - 19:00	443	7	273	1.001	334	12	314	987	1.988
19:00 - 20:00	261	10	229	738	322	14	264	879	1.617
20:00 - 21:00	136	3	192	526	195	10	225	664	1.190
21:00 - 22:00	125	4	149	432	164	7	213	602	1.034
Total	5.905	114	5.278	16.689	5.512	138	5.917	17.623	34.311

Elaboração: TTC

Tabela 2.16 – Resumo das contagens: Movimentos 1 a 3 - 2033

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 1				MOVIMENTO 2				MOVIMENTO 3			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	334	22	249	254	22	0	139	3	78	0	32	18
07:00 - 08:00	420	14	209	158	7	0	99	3	124	1	33	10
08:00 - 09:00	354	4	307	62	8	0	146	1	160	0	26	12
09:00 - 10:00	358	6	319	54	14	0	176	0	166	1	25	12
10:00 - 11:00	301	8	230	56	14	0	176	0	125	0	37	10
11:00 - 12:00	327	3	316	32	21	4	180	0	128	1	14	10
12:00 - 13:00	227	3	261	25	12	0	133	0	113	1	12	8
13:00 - 14:00	351	6	256	66	12	0	142	1	99	1	26	7
14:00 - 15:00	323	8	360	106	18	0	186	0	80	1	26	10
15:00 - 16:00	424	3	249	63	14	0	147	3	89	1	17	19
16:00 - 17:00	451	8	334	47	26	0	172	1	94	0	10	8
17:00 - 18:00	488	10	283	113	25	1	209	4	102	0	11	11
18:00 - 19:00	395	6	249	84	15	1	73	3	95	1	10	15
19:00 - 20:00	227	11	216	48	10	0	59	0	67	0	8	11
20:00 - 21:00	128	4	171	21	6	0	63	1	33	3	0	0
21:00 - 22:00	124	4	129	19	1	0	54	0	29	0	1	1
Total	5.232	118	4.139	1.208	224	7	2.154	21	1.582	14	288	162

Elaboração: TTC

Tabela 2.17 – Resumo das contagens: Movimentos 4 a 6 - 2033

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 4				MOVIMENTO 5				MOVIMENTO 6			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	7	0	0	0	59	1	7	12	191	7	336	74
07:00 - 08:00	4	0	0	0	91	1	8	14	499	10	435	117
08:00 - 09:00	4	0	3	0	114	1	12	3	387	10	311	18
09:00 - 10:00	4	0	1	0	132	1	17	7	418	14	333	39
10:00 - 11:00	1	0	0	0	153	0	28	11	389	4	315	29
11:00 - 12:00	6	0	0	0	129	0	29	8	323	8	316	33
12:00 - 13:00	6	0	0	0	128	1	28	6	348	3	314	44
13:00 - 14:00	1	0	0	0	140	0	29	19	348	10	268	33
14:00 - 15:00	4	0	1	1	161	1	32	8	252	11	358	28
15:00 - 16:00	3	0	0	1	198	3	36	17	239	7	326	59
16:00 - 17:00	6	0	0	1	205	1	44	15	272	10	366	44
17:00 - 18:00	6	0	0	1	195	1	28	17	429	12	305	127
18:00 - 19:00	7	0	0	1	153	1	21	3	310	12	312	106
19:00 - 20:00	4	0	0	0	95	1	12	8	327	18	264	54
20:00 - 21:00	7	0	0	0	44	0	7	1	206	10	219	22
21:00 - 22:00	0	0	0	3	32	1	4	8	176	8	212	19
Total	69	0	6	10	2.029	18	340	157	5.116	153	4.990	845

Elaboração: TTC

Tabela 2.18 – Resumo das contagens: Movimentos 7 a 9 - 2033

VOLUME POR MOVIMENTO												
Hora	MOVIMENTO 7 - ENTRADA NO ATERRO				MOVIMENTO 8 - SAÍDA DO ATERRO				MOVIMENTO 9			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	0	0	0	0	0	0	0	0	14	6	12	12
07:00 - 08:00	7	3	19	0	3	0	10	0	25	25	29	19
08:00 - 09:00	1	0	18	0	3	0	18	0	22	45	32	0
09:00 - 10:00	3	0	15	0	1	0	26	0	28	6	41	3
10:00 - 11:00	3	0	25	0	4	0	18	0	29	1	40	6
11:00 - 12:00	6	0	25	0	4	0	28	0	12	4	44	1
12:00 - 13:00	1	0	21	0	3	0	19	0	15	0	52	0
13:00 - 14:00	6	0	19	0	0	0	19	0	21	6	39	6
14:00 - 15:00	1	0	15	0	6	0	12	0	21	7	62	7
15:00 - 16:00	0	0	21	0	0	1	15	0	19	8	41	3
16:00 - 17:00	3	0	14	0	3	0	23	0	21	3	43	8
17:00 - 18:00	3	0	3	0	7	1	6	0	19	0	54	4
18:00 - 19:00	1	0	6	0	0	0	4	0	8	3	44	3
19:00 - 20:00	1	0	3	0	0	0	6	0	6	3	30	12
20:00 - 21:00	0	0	3	0	0	0	6	0	1	0	30	6
21:00 - 22:00	0	0	0	0	0	0	0	0	7	1	25	4
Total	36	3	205	0	33	3	209	0	267	117	618	94

Elaboração: TTC

Tabela 2.19 – Resumo das contagens: Movimentos 10 e 11 - 2033

VOLUME POR MOVIMENTO								
Hora	MOVIMENTO 10				MOVIMENTO 11			
	Auto	Ônibus	Caminhão	Moto	Auto	Ônibus	Caminhão	Moto
06:00 - 07:00	21	15	23	4	144	17	12	39
07:00 - 08:00	41	18	22	10	257	36	17	103
08:00 - 09:00	39	23	15	8	63	18	21	12
09:00 - 10:00	36	22	19	7	54	18	30	6
10:00 - 11:00	48	14	26	4	52	14	22	7
11:00 - 12:00	50	18	37	10	37	14	18	8
12:00 - 13:00	43	17	30	7	58	11	21	21
13:00 - 14:00	37	18	30	8	32	17	17	11
14:00 - 15:00	40	10	23	11	47	12	18	17
15:00 - 16:00	74	22	25	10	52	17	22	8
16:00 - 17:00	125	18	30	40	40	14	15	6
17:00 - 18:00	191	39	26	72	39	18	22	4
18:00 - 19:00	69	19	29	17	40	15	25	12
19:00 - 20:00	44	14	26	18	28	14	23	6
20:00 - 21:00	14	12	19	1	21	12	23	1
21:00 - 22:00	17	14	15	10	15	14	10	4
Total	887	292	398	235	978	259	315	264

Elaboração: TTC

Tabela 2.20 – Volume na Rodovia SP-055 - 2033

VOLUME POR MOVIMENTO									
Hora	PISTA OESTE - CUBATÃO				PISTA LESTE - GUARUJÁ				TOTAL
	Auto	Ônibus	Caminhão	VEIC. EQ.	Auto	Ônibus	Caminhão	VEIC. EQ.	VEIC. EQ.
06:00 - 07:00	409	23	395	1.245	292	7	506	1.318	2.563
07:00 - 08:00	513	15	316	1.176	630	11	567	1.786	2.962
08:00 - 09:00	472	6	462	1.407	554	10	483	1.539	2.947
09:00 - 10:00	499	7	510	1.534	598	15	534	1.696	3.230
10:00 - 11:00	466	8	433	1.350	528	4	528	1.593	2.943
11:00 - 12:00	472	7	526	1.537	472	14	510	1.520	3.057
12:00 - 13:00	362	4	422	1.215	473	4	459	1.400	2.615
13:00 - 14:00	502	6	426	1.366	459	11	436	1.354	2.720
14:00 - 15:00	498	10	576	1.670	349	12	570	1.513	3.183
15:00 - 16:00	633	6	432	1.508	343	8	490	1.339	2.846
16:00 - 17:00	677	10	550	1.797	392	10	548	1.506	3.303
17:00 - 18:00	703	12	520	1.768	556	14	526	1.634	3.402
18:00 - 19:00	556	8	343	1.257	420	15	395	1.239	2.497
19:00 - 20:00	327	12	288	927	404	18	332	1.103	2.030
20:00 - 21:00	171	4	241	660	245	12	282	834	1.494
21:00 - 22:00	157	6	187	542	206	8	267	757	1.299
Total	7.416	143	6.628	20.958	6.922	173	7.431	22.132	43.090

Elaboração: TTC

3. COMPOSIÇÃO DOS CENÁRIOS DE ANÁLISE

O impacto do empreendimento foi avaliado em diferentes cenários que representam as alterações no fluxo de passagem em diferentes fases.

Na Tabela 3.1 é apresentada a composição dos diferentes cenários de análise, totalizando 4 cenários.

Destaca-se que na contagem realizada, o empreendimento já operava como aterro, mas como é impossível verificar a quantidade de veículos com destino ou origem no Aterro dentro dos veículos contados, optou-se por acrescentar a demanda atual e prevista para o futuro sobre o fluxo atual sem reduzir a demanda possivelmente contada no levantamento, o que poderia gerar certa duplicidade de viagens, mantendo certa margem de segurança quanto ao impacto futuro.

Tabela 3.1 – Composição dos Cenários de Análise do Impacto

Composição dos Cenários		Ano	Cenários
Situação Atual	Fluxo de passagem no ano de realização das Contagens + Viagens do Aterro	2019	Cenário 1
Implantação	Fluxo de passagem no ano de início de implantação + Viagens do Aterro + Viagens dos veículos da Obra	2020	Cenário 2
Operação Futura	Fluxo de passagem no ano de conclusão da implantação + Viagens da URE	2023	Cenário 3
Operação Futura	Fluxo de passagem 10 anos após início da operação total + Viagens da URE	2033	Cenário 4

Fonte: TTC

Considerou-se que a implantação da URE terá início no ano de 2020 e será concluída em 36 meses, ou seja, até 2023. Além disso, foi feita a análise para 10 anos após a o início da operação total da URE.

4. ESTIMATIVA DE DEMANDA

No presente capítulo são abordadas as seguintes etapas:

- Geração de viagens;
- Divisão Modal;
- Distribuição temporal.

Há ainda a etapa de distribuição espacial das viagens, apresentada no Capítulo 1 para cada etapa de operação.

4.1. Geração de Viagens

As viagens geradas pelo empreendimento foram obtidas através das contagens veiculares classificadas e informações da operação atual fornecidas pelo empreendedor.

Com base nas informações da Terrestre Ambiental, obteve-se a quantidade de viagens por município na hora de maior fluxo no aterro. Essa proporção foi aplicada ao total de veículos da hora mais carregada obtida na contagem veicular classificada.

Na Tabela 4.1 são apresentados os volumes de entrada e saída de veículos utilizados na operação do empreendimento como aterro ou URE.

Tabela 4.1 – Volumes de Entrada e Saída de Veículos da Operação

HORA CRÍTICA			
DESTINO	ENTRADA	SAÍDA	PROPORÇÃO
Bertioga	1	1	4%
Guarujá	1	2	8%
Santos	1	2	8%
Cubatão	7	6	35%
São Vicente	5	5	27%
Praia Grande	2	2	12%
Mongaguá	1	2	8%
Total	18	20	100%

Fonte: **Ribeirão Energia** - Elaboração: **TTC**

O volume total de caminhões na hora mais carregada, obtido na contagem, representa 5% do total de caminhões da hora mais carregada da rodovia.

Considera-se a troca de turno a hora de maior fluxo de veículos de transporte de funcionários, conforme apresentado na Tabela 4.2.

Tabela 4.2 – Volumes de Entrada e Saída de Veículos de Transporte de Funcionários

HORA CRÍTICA		
VEÍCULO	ENTRADA	SAÍDA
Ônibus	2	2
Van	1	1
Carro	5	5
Total	8	8

Elaboração: TTC

Na Tabela 4.3 são apresentados os volumes de entrada e saída dos veículos da obra, utilizados durante a fase implantação da URE.

Tabela 4.3 – Volumes de Entrada e Saída de Veículos de Obra

VIAGENS POR DIA				
ROTA	MOBILIZAÇÃO		DESMOBILIZAÇÃO	
	LEVE	PESADO	LEVE	PESADO
ROTA 1	0	0	0	0
ROTA 2	1	3	1	3
ROTA 3	0	0	0	0
ROTA 4	0	0	0	0
ROTA 5	0	0	0	0
ROTA 5 A	4	5	4	5
Total	5	8	5	8

Fonte: *Ribeirão Energia* - Elaboração: TTC

A estimativa de viagens durante a fase de implantação da URE foi feita a partir das informações fornecidas pelo empreendedor e apresentadas no item 1.4.

Uma vez que as viagens de máquinas e equipamentos, na fase de implantação, resultavam em menos de uma viagem por dia, foram desconsideradas deste estudo.

4.2. Divisão Modal

Na Tabela 4.4 apresenta-se a divisão modal da demanda gerada pelo empreendimento objeto deste estudo.

Tabela 4.4 – Divisão Modal para Estudo de Tráfego

Veículos	%
Leve	27,50 %
Pesado	72,50 %

Elaboração: TTC

4.3. Distribuição Temporal

Como não há informações precisas da flutuação horária dos fluxos de entrada e saída dos veículos de coleta e dos veículos que serão utilizados durante a fase de implantação, considerou-se a situação hipotética mais crítica: que toda a demanda estimada irá circular na hora de maior fluxo da rodovia.

5. ANÁLISE DO IMPACTO

A capacidade do viário existente é testada por meio da estimativa dos níveis de serviço mediante dois conjuntos de dados, os quais consideram a soma dos volumes gerados pelo empreendimento (*CAPÍTULO 4 - ESTIMATIVA DE DEMANDA*) ao fluxo de passagem atual e o previsto para o futuro (*CAPÍTULO 2 – FLUXO DE PASSAGEM*).

A partir dessas informações avalia-se a capacidade de suporte do dispositivo utilizado pelo empreendimento para os movimentos de retorno nos cenários estudados conforme *Capítulo 3– COMPOSIÇÃO DOS CENÁRIOS DE ANÁLISE*.

As análises foram feitas utilizando a densidade para a rodovia, levando em consideração os intervalos dos níveis de serviço recomendados pelo *HCM (Highway Capacity Manual)* na edição 2010.

A análise do desempenho do sistema viário é realizada utilizando o conceito de Nível de Serviço (NS), que é uma grandeza associada à sensação de conforto percebido pelo condutor ao transitar por uma via, sendo dividido em seis classes que variam de A até F, conforme ilustrado na Figura 5.1.

Figura 5.1 – Imagem ilustrativa dos diferentes Níveis de Serviço (NS)



Nível de Serviço A (ótimo)



Nível de Serviço B (bom)



**Nível de Serviço C
(aceitável)**



Nível de Serviço D (regular)



Nível de Serviço E (ruim)



Nível de Serviço F (péssimo)

Fonte: HCM(2010)

A presença de interseções em nível localizadas dentro ou mesmo fora dos limites da faixa de domínio é capaz de afetar a operação da rodovia. Para analisar a qualidade da operação de

interseções em nível próximas a rodovias, é utilizado o *software SIDRA Intersection (Signalised and unsignalised Intersection Design and Research Aid)*, utilizando a metodologia do *HCM2010*.

5.1. Segmentos Analisados

Foram analisados os segmentos onde os veículos circulam para acessar ou deixar a área do empreendimento, conforme apresentado na Figura 5.2.

Figura 5.2 – Segmentos Estudados



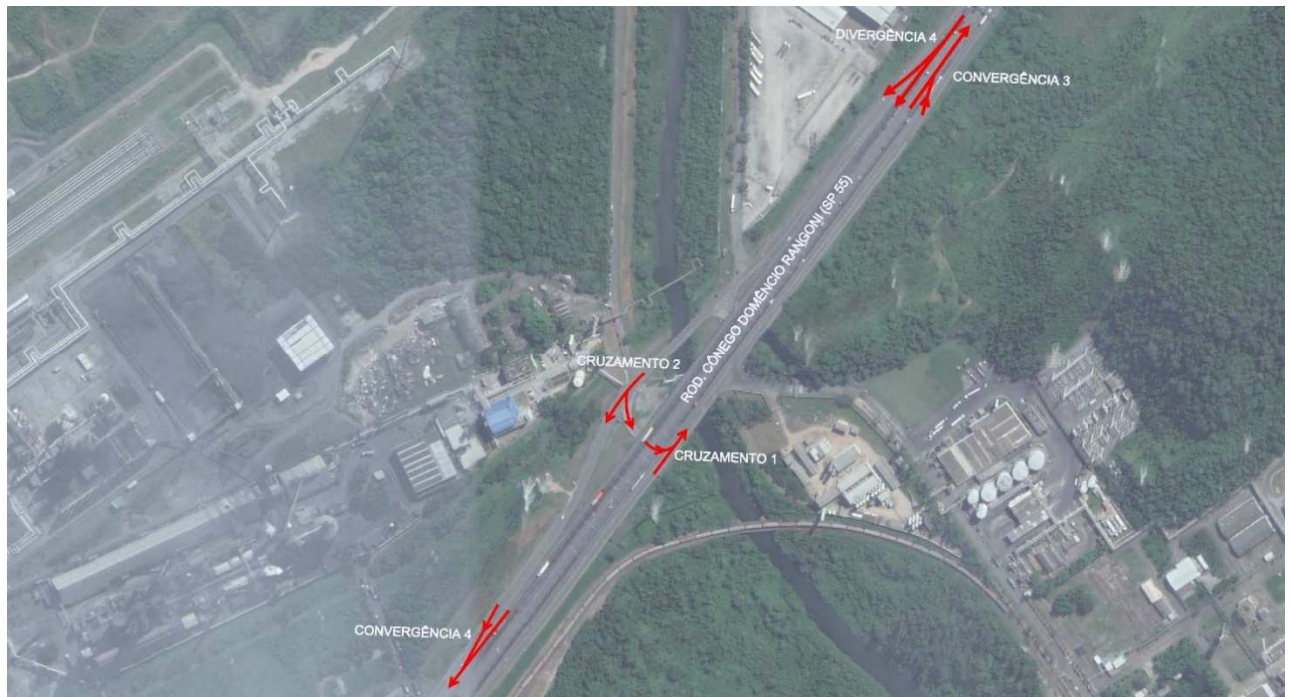
Fonte: TTC

Figura 5.3 – Segmentos Estudados



Fonte: TTC

Figura 5.4 – Segmentos Estudados



Fonte: TTC

Foram analisadas 4 convergências, 4 divergências, 1 entrelaçamento e 2 cruzamentos, além das seções da rodovia, antes e após o acesso ao empreendimento.

Ressalta-se que o volume de veículos de coleta de resíduos, já está incorporado nos dados das contagens veiculares classificadas. Entretanto, devido à dificuldade em determinar um horário preciso de sua circulação, o volume dos veículos de coleta foi somado ao fluxo de passagem obtido nas pesquisas.

5.2. Resultados

Na Tabela 5.1 a seguir são apresentados os resultados obtidos aplicando-se a metodologia do HCM2010, para cada um dos cenários analisados.

Tabela 5.1 – Níveis de Serviço

PONTO	CENÁRIO 1	CENÁRIO 2	CENÁRIO 3	CENÁRIO 4
Convergência 1	B	B	B	B
Convergência 2	B	B	B	B
Convergência 3	A	A	A	A
Convergência 4	A	A	A	A
Divergência 1	A	A	B	B
Divergência 2	A	A	A	A
Divergência 3	A	A	A	B
Divergência 4	A	B	B	B
Cruzamento 1	A	A	A	A
Cruzamento 2	A	A	A	A
Entrelaçamento	A	A	A	A
Seção 1 - Sentido Guarujá (Sent.1)	A	B	B	B
Seção 1 - Sentido Cubatão (Sent. 2)	B	B	B	B
Seção 2 - Sentido Guarujá (Sent.1)	B	B	B	C
Seção 2 - Sentido Cubatão (Sent. 2)	B	B	B	C

Fonte: TTC

Observa-se que, para todos os cenários analisados, todos os pontos apresentaram Níveis de Serviço “A” (ótimo), “B” (bom) ou “C” (aceitável), ou seja, todos os pontos da rodovia por onde os veículos do empreendimento irão circular (área de influência direta) operam dentro dos padrões qualidade ideais, mesmo para os anos futuros.

Ressalta-se que o aterro já está em operação e seu volume já estava contemplado no fluxo de passagem obtido nas contagens veiculares classificadas, ou seja, o empreendimento só provocará um incremento real no volume de tráfego durante a implantação da URE. Entretanto, devido à incerteza da distribuição das viagens da operação do empreendimento, seu maior volume horário foi acrescentado às contagens em todos os cenários.

Os relatórios com os dados de saída dos softwares HCS 2010 e Sidra Intersection são apresentados no capítulo 7 ANEXOS.

6. CONCLUSÃO

Neste documento foram avaliadas as condições de acesso à Unidade de Recuperação de Energia – URE Valoriza Energia, que será implantada no km 254,9 da Rodovia Cônego Domênico Rangoni, na pista leste, no município de Santos, em área lindeira ao aterro CGR Terrestre.

Foram levantadas as características da implantação e da operação do empreendimento, incluindo frota, número de viagens e rotas utilizadas pelos veículos do empreendimento.

Para caracterizar o tráfego de passagem, foram realizadas contagens veiculares classificadas no mês de junho de 2019, que foram extrapoladas para o ano de 2020 (ano de início de implantação do empreendimento), para o ano de 2023 (ano do fim da implantação do empreendimento) e 2033 (10 anos após a conclusão da implantação).

A partir disso, foram construídos cenários com base na demanda gerada, informações da operação atual do empreendimento e das características da operação futura. Em seguida foram estimadas as distribuições espaciais, com base nas rotas fornecidas pelo empreendedor e avaliadas as condições de tráfego para cada um dos cenários.

A avaliação do desempenho do sistema viário foi realizada utilizando o *HCM (Highway Capacity Manual)* e *Sidra Intersection* que contêm conceitos, diretrizes e procedimentos computacionais para calcular a capacidade e qualidade das malhas viárias.

6.1. Avaliação Final

Com a identificação das rotas utilizadas pelos veículos do empreendimento na área diretamente afetada (ADA), foi realizada a análise dos pontos de convergência, divergência, entrelaçamento, cruzamento e das seções da Rodovia Cônego Domênico Rangoni.

A rodovia que dá acesso à área do futuro empreendimento apresenta uma grande movimentação de veículos de carga, devido à intensa atividade portuária da região, sendo que a contribuição do empreendimento é de apenas 5% considerando o trecho de maior concentração das viagens, área de influência direta, localizado entre os dispositivos de retorno para ambos sentidos da rodovia. Vale destacar a possível duplicidade da demanda, uma vez que a contagem pode ter contabilizado os veículos atuais que se dirigem ao aterro e que serão utilizados pela URE e que não existem dados de distribuição temporal diária rotineira das operações futuras e das viagens para implantação, portanto foram consideradas viagens em uma mesma hora pico, conflitante com a hora pico da rodovia, o que gera uma grande margem de segurança nos resultados.

Os resultados obtidos com a aplicação da metodologia do *HCM 2010* indica que com o acréscimo do volume de tráfego do empreendimento, na situação atual, durante a implantação da URE e na fase de operação da URE, o sistema viário do entorno irá operar em condições de tráfego adequadas inclusive no cenário previsto para o ano de 2033, considerando inclusive o crescimento vegetativo com base em dados da ABCR.

7. ANEXOS

ANEXO 01

Tabulação das Contagens

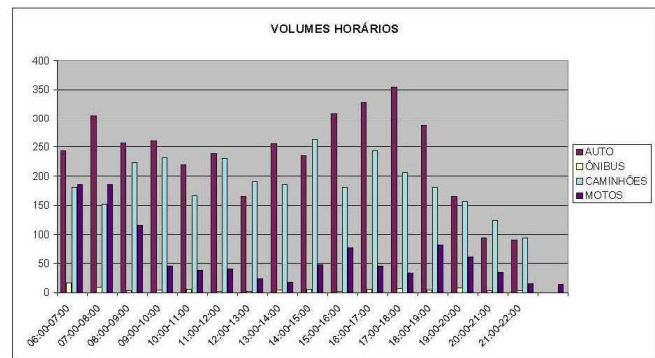
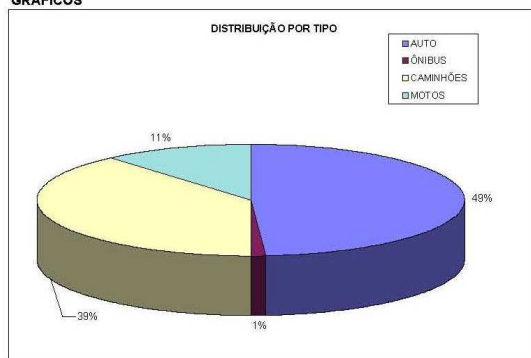
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248						SENTIDO: 1 - SÃO PAULO						PONTO: 1	
DATA: 25/6/2019						TEMPO: BOM		TRAFEGO: NORMAL					
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	49	5	62	38	154	14:15	64	3	66	27	160		
06:30	69	5	33	42	149	14:30	61	1	60	29	151		
06:45	67	3	48	58	176	14:45	55	1	65	13	134		
07:00	58	3	38	47	146	15:00	55	1	71	8	135		
07:15	96	2	38	32	168	15:15	82	0	57	4	143		
07:30	69	4	32	32	137	15:30	65	0	59	17	141		
07:45	76	3	39	32	150	15:45	68	1	46	8	123		
08:00	64	1	43	19	127	16:00	93	1	13	17	130		
08:15	57	0	43	11	111	16:15	86	2	62	4	154		
08:30	63	1	54	8	126	16:30	72	0	61	9	142		
08:45	67	0	73	13	153	16:45	68	2	64	12	146		
09:00	70	2	53	13	138	17:00	102	2	56	9	169		
09:15	54	1	66	7	128	17:15	83	1	33	12	129		
09:30	67	2	51	9	129	17:30	88	3	44	21	156		
09:45	67	0	52	14	133	17:45	78	3	47	31	159		
10:00	72	1	63	9	145	18:00	106	0	82	18	206		
10:15	66	2	55	16	139	18:15	79	1	44	21	145		
10:30	39	1	26	10	76	18:30	74	0	43	16	135		
10:45	62	0	38	8	108	18:45	79	3	38	15	135		
11:00	52	3	48	7	110	19:00	55	0	56	7	118		
11:15	58	0	66	4	128	19:15	34	2	57	16	109		
11:30	63	0	66	4	133	19:30	53	4	33	11	101		
11:45	66	1	47	12	126	19:45	49	2	39	5	95		
12:00	51	1	51	3	106	20:00	29	0	28	3	60		
12:15	27	1	41	0	69	20:15	38	1	42	6	87		
12:30	25	0	44	1	70	20:30	24	0	26	3	53		
12:45	65	0	66	8	139	20:45	17	0	28	5	50		
13:00	48	1	39	9	97	21:00	14	2	28	1	45		
13:15	55	2	53	7	117	21:15	16	2	24	3	45		
13:30	53	0	42	8	103	21:30	12	0	18	1	31		
13:45	72	0	52	15	139	21:45	9	1	26	4	40		
14:00	75	2	39	18	134	22:00	53	0	26	6	85		
SUBTOTAL					4064	SUBTOTAL					3712		
TOTAL GERAL					7776								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	243	16	181	185	625	14:00-15:00	235	6	262	77	580
07:00-08:00	305	10	152	115	582	15:00-16:00	308	2	181	46	537
08:00-09:00	257	3	223	45	528	16:00-17:00	328	6	243	34	611
09:00-10:00	260	4	232	39	535	17:00-18:00	355	7	206	82	650
10:00-11:00	219	6	167	41	433	18:00-19:00	287	4	181	61	533
11:00-12:00	238	2	230	23	493	19:00-20:00	165	8	157	35	365
12:00-13:00	165	2	190	18	375	20:00-21:00	93	3	124	15	235
13:00-14:00	255	4	188	48	493	21:00-22:00	90	3	94	14	201
SUBTOTAL	1942	47	1561	514	4064	SUBTOTAL	1861	39	1448	364	3712
TOTAL GERAL	3803	86	3009	878	7776						

GRÁFICOS



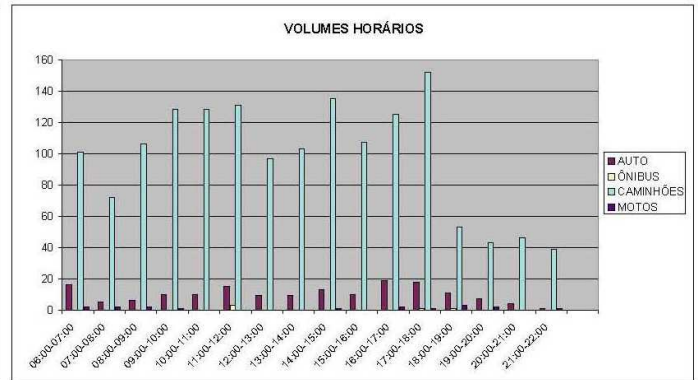
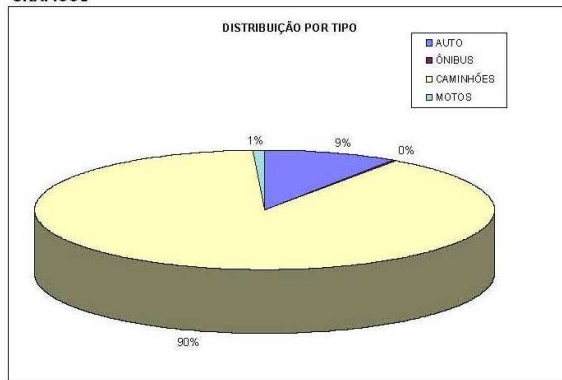
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248						SENTIDO: 2 - RETORNO PARA SÃO PAULO					PONTO: 1
DATA: 25/06/2019						TEMPO		BOM		TRÁFEGO: NORMAL	
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:15	8	0	26	0	34	14:15	1	0	37	0	38
06:30	2	0	25	2	29	14:30	5	0	47	0	52
06:45	4	0	28	0	32	14:45	5	0	32	0	37
07:00	2	0	22	0	24	15:00	2	0	19	0	21
07:15	2	0	15	1	18	15:15	2	0	22	0	24
07:30	2	0	15	0	17	15:30	0	0	27	0	27
07:45	1	0	22	0	23	15:45	5	0	33	1	39
08:00	0	0	20	1	21	16:00	3	0	25	1	29
08:15	0	0	18	0	18	16:15	4	0	24	0	28
08:30	1	0	28	1	30	16:30	3	0	28	0	31
08:45	1	0	22	0	23	16:45	5	0	31	0	36
09:00	4	0	38	0	42	17:00	7	0	42	1	50
09:15	1	0	25	0	26	17:15	4	0	39	0	43
09:30	4	0	21	0	25	17:30	5	1	66	0	72
09:45	2	0	32	0	34	17:45	2	0	21	0	23
10:00	3	0	50	0	53	18:00	7	0	26	3	36
10:15	0	0	45	0	45	18:15	7	0	16	1	24
10:30	3	0	35	0	38	18:30	2	1	17	1	21
10:45	5	0	26	0	31	18:45	1	0	12	0	13
11:00	2	0	22	0	24	19:00	1	0	8	0	9
11:15	2	1	21	0	24	19:15	2	0	16	0	18
11:30	0	2	8	0	10	19:30	2	0	7	0	9
11:45	9	0	76	0	85	19:45	1	0	16	0	17
12:00	4	0	26	0	30	20:00	2	0	4	0	6
12:15	5	0	28	0	33	20:15	1	0	7	0	8
12:30	3	0	22	0	25	20:30	0	0	8	0	8
12:45	0	0	24	0	24	20:45	2	0	17	0	19
13:00	1	0	23	0	24	21:00	1	0	14	1	16
13:15	3	0	26	0	29	21:15	1	0	13	0	14
13:30	1	0	30	0	31	21:30	0	0	10	0	10
13:45	4	0	17	1	22	21:45	0	0	10	0	10
14:00	1	0	30	0	31	22:00	0	0	6	0	6
SUBTOTAL					955	SUBTOTAL					794
TOTAL GERAL					1749						

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	16	0	101	2	119	14:00-15:00	13	0	135	0	148
07:00-08:00	5	0	72	2	79	15:00-16:00	10	0	107	2	119
08:00-09:00	6	0	106	1	113	16:00-17:00	19	0	125	1	145
09:00-10:00	10	0	128	0	138	17:00-18:00	18	1	152	3	174
10:00-11:00	10	0	128	0	138	18:00-19:00	11	1	53	2	67
11:00-12:00	15	3	131	0	149	19:00-20:00	7	0	43	0	50
12:00-13:00	9	0	97	0	106	20:00-21:00	4	0	46	1	51
13:00-14:00	9	0	103	1	113	21:00-22:00	1	0	39	0	40
SUBTOTAL	80	3	886	6	955	SUBTOTAL	83	2	700	9	794
TOTAL GERAL	163	5	1566	15	1749						

GRÁFICOS



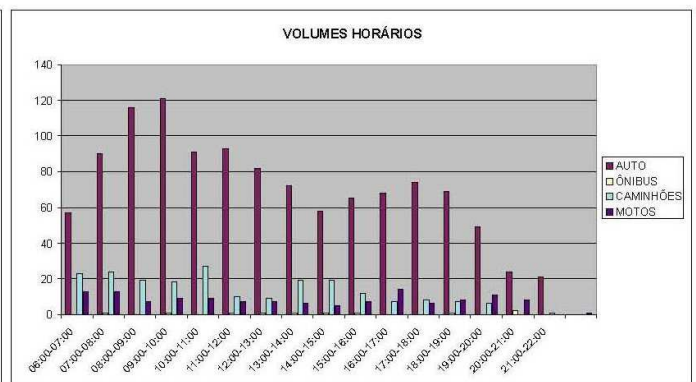
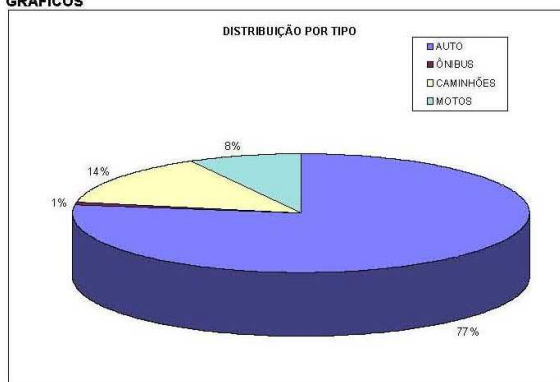
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248						SENTIDO: 3 - BERTIOGA /SÃO SEBASTIÃO					PONTO: 1
DATA: 26/6/2019						TEMPO		BOM		TRÁFEGO: NORMAL	
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:15	8	0	3	1	10	14:15	14	0	7	2	23
06:30	22	0	4	2	28	14:30	11	0	4	2	17
06:45	19	0	10	4	33	14:45	16	0	4	1	21
07:00	10	0	6	6	22	15:00	17	1	4	2	24
07:15	21	0	5	4	30	15:15	20	0	4	1	25
07:30	19	0	6	1	26	15:30	12	1	4	10	27
07:45	23	1	11	0	35	15:45	19	0	3	2	24
08:00	27	0	2	2	31	16:00	14	0	1	1	16
08:15	22	0	5	2	29	16:15	18	0	0	2	20
08:30	23	0	5	0	28	16:30	17	0	3	2	22
08:45	37	0	2	3	42	16:45	16	0	3	2	21
09:00	34	0	7	4	45	17:00	17	0	1	0	18
09:15	25	0	2	1	28	17:15	13	0	1	1	15
09:30	40	0	4	3	47	17:30	17	0	4	1	22
09:45	18	0	9	2	29	17:45	25	0	1	4	30
10:00	38	1	3	3	45	18:00	19	0	2	2	23
10:15	19	0	9	0	28	18:15	25	0	3	2	30
10:30	26	0	9	3	38	18:30	10	0	2	2	14
10:45	24	0	4	2	30	18:45	20	1	1	4	26
11:00	22	0	5	2	29	19:00	14	0	1	3	18
11:15	28	0	1	0	29	19:15	13	0	0	2	15
11:30	23	0	3	0	26	19:30	13	0	1	0	14
11:45	20	0	4	4	28	19:45	10	0	3	1	14
12:00	22	1	2	3	28	20:00	13	0	2	5	20
12:15	23	0	2	1	26	20:15	5	2	0	0	7
12:30	16	1	1	2	20	20:30	9	0	0	0	9
12:45	20	0	4	2	26	20:45	9	0	0	0	9
13:00	23	0	2	1	26	21:00	1	0	0	0	1
13:15	20	0	9	2	31	21:15	4	0	1	0	5
13:30	28	1	5	1	35	21:30	5	0	0	1	6
13:45	6	0	3	1	10	21:45	3	0	0	0	3
14:00	18	0	2	1	21	22:00	9	0	0	0	9
SUBTOTAL					939	SUBTOTAL					548
TOTAL GERAL					1487						

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	57	0	23	13	93	14:00-15:00	58	1	19	7	85
07:00-08:00	90	1	24	7	122	15:00-16:00	65	1	12	14	92
08:00-09:00	116	0	19	9	144	16:00-17:00	68	0	7	6	81
09:00-10:00	121	1	18	9	149	17:00-18:00	74	0	8	8	90
10:00-11:00	91	0	27	7	125	18:00-19:00	68	1	7	11	88
11:00-12:00	93	1	10	7	111	19:00-20:00	49	0	6	8	63
12:00-13:00	82	1	9	6	98	20:00-21:00	24	2	0	0	26
13:00-14:00	72	1	19	5	97	21:00-22:00	21	0	1	1	23
SUBTOTAL	722	5	149	63	939	SUBTOTAL	428	5	80	55	548
TOTAL GERAL	1150	10	209	118	1487						

GRÁFICOS



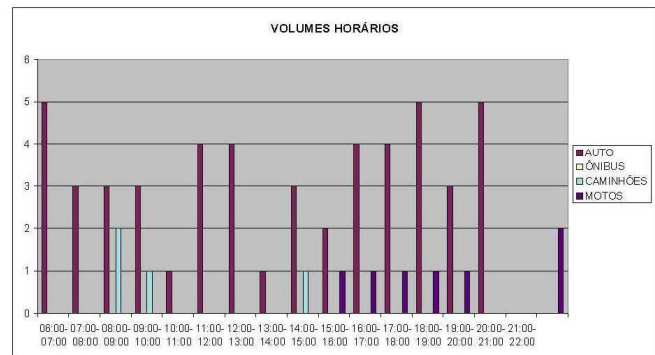
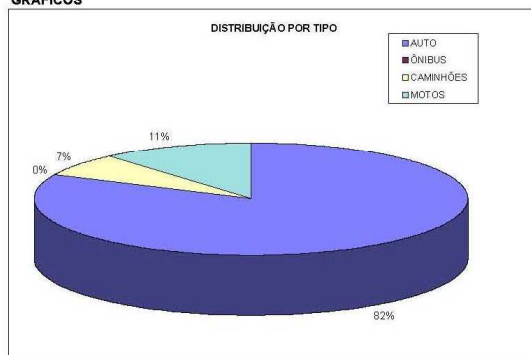
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA:		RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248				SENTIDO:		4- RETORNO PARA GUARUJÁ				PONTO:		1		
DATA:		25/6/2019				TEMPO:		BOM				TRAFEGO:		NORMAL		
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL					
06:15	1	0	0	0	1	14:15	1	0	0	0	1					
06:30	3	0	0	0	3	14:30	1	0	0	0	1					
06:45	0	0	0	0	0	14:45	1	0	0	0	1					
07:00	1	0	0	0	1	15:00	0	0	1	0	1					
07:15	1	0	0	0	1	15:15	0	0	0	0	0					
07:30	0	0	0	0	0	15:30	2	0	0	0	2					
07:45	0	0	0	0	0	15:45	0	0	0	0	1					
08:00	2	0	0	0	2	16:00	0	0	0	0	0					
08:15	0	0	0	0	0	16:15	1	0	0	0	1					
08:30	0	0	0	0	0	16:30	1	0	0	0	1					
08:45	2	0	1	0	3	16:45	1	0	0	0	1					
09:00	1	0	1	0	2	17:00	1	0	0	0	1					
09:15	0	0	1	0	1	17:15	0	0	0	0	0					
09:30	1	0	0	0	1	17:30	2	0	0	0	1					
09:45	1	0	0	0	1	17:45	0	0	0	0	0					
10:00	1	0	0	0	1	18:00	2	0	0	0	2					
10:15	1	0	0	0	1	18:15	2	0	0	0	2					
10:30	0	0	0	0	0	18:30	1	0	0	0	1					
10:45	0	0	0	0	0	18:45	0	0	0	0	0					
11:00	0	0	0	0	0	19:00	2	0	0	0	2					
11:15	1	0	0	0	1	19:15	2	0	0	0	2					
11:30	1	0	0	0	1	19:30	0	0	0	0	0					
11:45	2	0	0	0	2	19:45	1	0	0	0	1					
12:00	0	0	0	0	0	20:00	0	0	0	0	0					
12:15	2	0	0	0	2	20:15	2	0	0	0	2					
12:30	0	0	0	0	0	20:30	0	0	0	0	0					
12:45	0	0	0	0	0	20:45	2	0	0	0	2					
13:00	2	0	0	0	2	21:00	1	0	0	0	1					
13:15	0	0	0	0	0	21:15	0	0	0	0	0					
13:30	1	0	0	0	1	21:30	0	0	0	0	1					
13:45	0	0	0	0	0	21:45	0	0	0	0	1					
14:00	0	0	0	0	0	22:00	0	0	0	0	0					
SUBTOTAL					27	SUBTOTAL					34					
TOTAL GERAL					61											

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	5	0	0	0	5	14:00-15:00	3	0	1	1	5
07:00-08:00	3	0	0	0	3	15:00-16:00	2	0	0	1	3
08:00-09:00	3	0	2	0	5	16:00-17:00	4	0	0	1	5
09:00-10:00	3	0	1	0	4	17:00-18:00	4	0	0	1	5
10:00-11:00	1	0	0	0	1	18:00-19:00	5	0	0	1	6
11:00-12:00	4	0	0	0	4	19:00-20:00	3	0	0	0	3
12:00-13:00	4	0	0	0	4	20:00-21:00	5	0	0	0	5
13:00-14:00	1	0	0	0	1	21:00-22:00	0	0	0	2	2
SUBTOTAL	24	0	3	0	27	SUBTOTAL	26	0	1	7	34
TOTAL GERAL	50	0	4	7	61						

GRÁFICOS



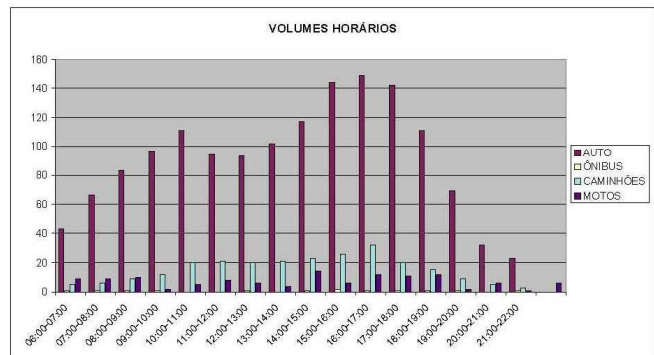
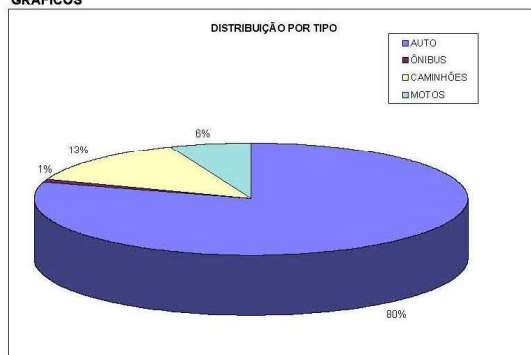
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248						SENTIDO: 5- VEM DE BERTIOGA SENTIDO SÃO PAULO						PONTO: 1	
DATA: 25/6/2019						TEMPO: BOM						TRÁFEGO: NORMAL	
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	7	0	1	2	10	14:15	33	0	4	2	39		
06:30	9	1	3	3	16	14:30	20	1	8	2	31		
06:45	10	0	0	1	11	14:45	38	0	5	2	45		
07:00	17	0	1	3	21	15:00	26	0	6	0	32		
07:15	18	0	2	2	22	15:15	44	2	8	3	57		
07:30	16	0	1	3	20	15:30	28	0	5	1	34		
07:45	16	1	0	2	19	15:45	43	0	4	4	51		
08:00	16	0	3	3	22	16:00	29	0	9	4	42		
08:15	12	0	5	0	17	16:15	34	0	4	4	42		
08:30	26	0	0	1	27	16:30	52	0	11	1	64		
08:45	20	1	2	0	23	16:45	34	1	11	5	51		
09:00	25	0	2	1	28	17:00	29	0	6	1	36		
09:15	29	0	1	1	31	17:15	42	1	7	4	54		
09:30	28	0	4	0	32	17:30	36	0	4	2	42		
09:45	20	1	1	3	25	17:45	33	0	4	2	39		
10:00	19	0	6	1	26	18:00	31	0	5	4	40		
10:15	42	0	13	5	60	18:15	29	1	2	-2	30		
10:30	9	0	4	0	13	18:30	24	0	1	0	25		
10:45	33	0	1	1	35	18:45	34	0	7	2	43		
11:00	27	0	2	2	31	19:00	24	0	5	2	31		
11:15	26	0	6	2	34	19:15	22	1	2	2	27		
11:30	32	0	5	2	39	19:30	14	0	1	1	16		
11:45	14	0	5	1	20	19:45	22	0	3	1	26		
12:00	22	0	5	1	28	20:00	11	0	3	2	16		
12:15	26	1	6	2	35	20:15	4	0	1	1	6		
12:30	11	0	6	0	17	20:30	11	0	2	0	13		
12:45	33	0	6	1	40	20:45	8	0	2	0	10		
13:00	23	0	2	1	26	21:00	9	0	0	0	9		
13:15	18	0	5	1	24	21:15	8	0	0	1	9		
13:30	18	0	5	3	26	21:30	8	0	2	1	11		
13:45	35	0	7	5	47	21:45	2	1	0	1	4		
14:00	31	0	4	5	40	22:00	5	0	1	3	9		
SUBTOTAL					865	SUBTOTAL					984		
TOTAL GERAL					1849								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	43	1	5	9	58	14:00-15:00	117	1	23	6	147
07:00-08:00	66	1	6	10	83	15:00-16:00	144	2	26	12	184
08:00-09:00	83	1	9	2	95	16:00-17:00	149	1	32	11	193
09:00-10:00	96	1	12	5	114	17:00-18:00	142	1	20	12	175
10:00-11:00	111	0	20	8	139	18:00-19:00	111	1	15	2	129
11:00-12:00	94	0	21	6	121	19:00-20:00	69	1	9	6	85
12:00-13:00	93	1	20	4	118	20:00-21:00	32	0	5	1	38
13:00-14:00	102	0	21	14	137	21:00-22:00	23	1	3	6	33
SUBTOTAL	688	5	114	58	865	SUBTOTAL	787	8	133	56	994
TOTAL GERAL	1475	13	247	114	1849						

GRÁFICOS



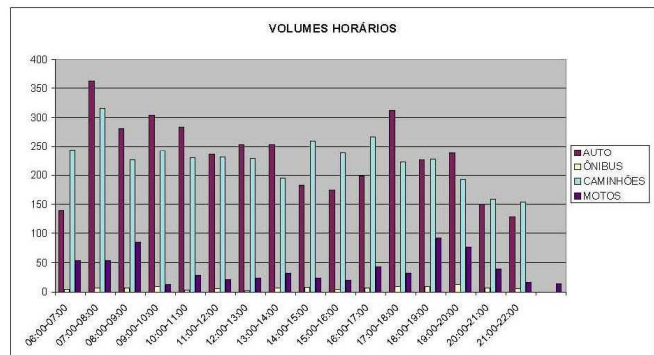
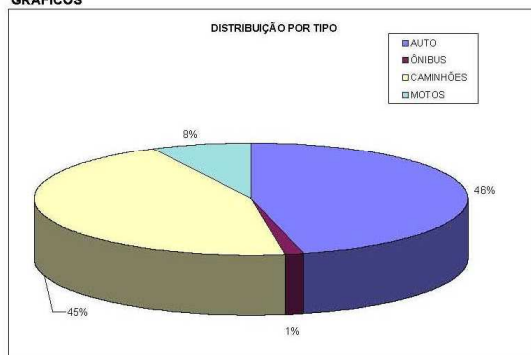
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248						SENTIDO: 6 - GUARUJÁ						PONTO: 1	
DATA: 25/6/2019						TEMPO: BOM						TRÁFEGO: NORMAL	
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	26	1	88	2	117	14:15	39	4	65	4	112		
06:30	33	2	63	10	108	14:30	40	1	85	1	127		
06:45	33	1	42	15	91	14:45	40	1	63	5	109		
07:00	47	1	51	27	126	15:00	64	2	47	10	123		
07:15	61	0	66	14	141	15:15	41	0	73	4	118		
07:30	92	2	76	12	182	15:30	45	0	54	11	110		
07:45	95	1	67	41	204	15:45	50	2	57	13	122		
08:00	115	4	107	18	244	16:00	38	3	53	15	109		
08:15	58	4	34	2	98	16:15	47	1	52	3	103		
08:30	65	1	62	6	134	16:30	22	1	52	11	86		
08:45	62	1	66	3	132	16:45	70	3	81	10	164		
09:00	96	1	64	2	163	17:00	59	2	81	8	150		
09:15	78	3	98	2	181	17:15	45	1	70	39	165		
09:30	70	2	6	11	89	17:30	83	3	38	15	139		
09:45	75	3	72	2	152	17:45	75	4	52	21	152		
10:00	81	2	68	13	162	18:00	109	1	62	17	189		
10:15	67	0	46	4	117	18:15	32	1	59	16	108		
10:30	86	1	63	2	152	18:30	69	2	51	21	143		
10:45	58	1	55	6	120	18:45	63	3	59	18	143		
11:00	72	1	65	9	147	19:00	61	3	58	22	144		
11:15	63	0	53	8	124	19:15	50	0	70	12	132		
11:30	50	2	78	4	134	19:30	71	8	56	7	142		
11:45	70	3	51	8	132	19:45	64	1	32	14	111		
12:00	52	1	48	4	105	20:00	53	4	34	6	97		
12:15	65	0	49	5	119	20:15	30	4	37	1	72		
12:30	56	0	53	7	116	20:30	47	1	52	5	105		
12:45	59	1	61	9	130	20:45	30	0	47	4	81		
13:00	73	1	65	11	150	21:00	43	2	23	6	74		
13:15	61	0	42	2	105	21:15	30	1	35	10	76		
13:30	61	1	41	7	110	21:30	31	0	41	2	74		
13:45	55	3	46	15	119	21:45	38	2	44	0	84		
14:00	76	3	66	0	145	22:00	29	3	34	2	68		
SUBTOTAL					4349	SUBTOTAL					3722		
TOTAL GERAL					8071								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	139	5	244	54	442	14:00-15:00	183	8	260	20	471
07:00-08:00	383	7	316	85	771	15:00-16:00	174	5	237	43	459
08:00-09:00	281	7	226	13	527	16:00-17:00	198	7	266	32	503
09:00-10:00	304	10	242	28	584	17:00-18:00	312	9	222	92	635
10:00-11:00	283	3	229	21	536	18:00-19:00	225	9	227	77	538
11:00-12:00	235	6	230	24	495	19:00-20:00	238	13	192	39	482
12:00-13:00	253	2	228	32	515	20:00-21:00	150	7	159	16	332
13:00-14:00	253	7	195	24	479	21:00-22:00	128	6	154	14	302
SUBTOTAL	2111	47	1910	281	4349	SUBTOTAL	1608	64	1717	333	3722
TOTAL GERAL	3719	111	3627	614	8071						

GRÁFICOS



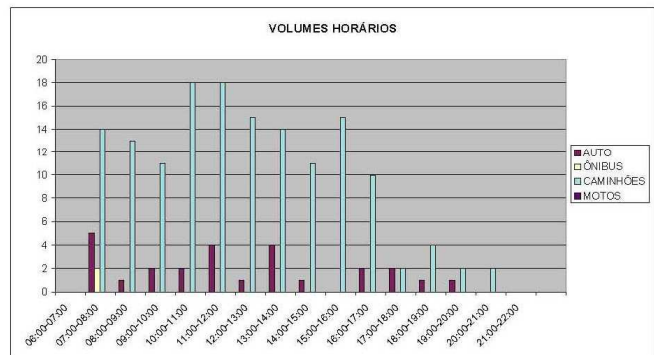
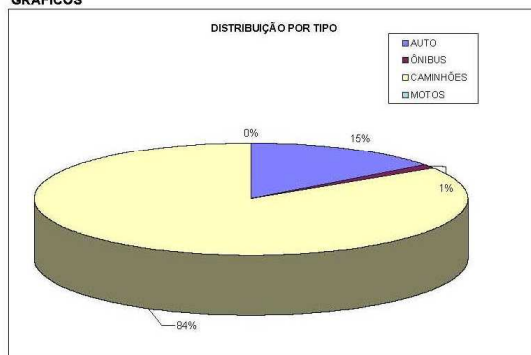
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: (via interna para Aterro Sanitário) RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 253						SENTIDO: 7 - ATERRRO						PONTO: 2	
DATA: 27/6/2019						TEMPO		GAROA		TRÁFEGO: NORMAL			
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	0	0	0	0	0	14:15	0	0	0	0	0		
06:30	0	0	0	0	0	14:30	0	0	5	0	5		
06:45	0	0	0	0	0	14:45	0	0	1	0	1		
07:00	0	0	0	0	0	15:00	1	0	5	0	6		
07:15	2	2	5	0	9	15:15	0	0	6	0	6		
07:30	0	0	1	0	1	15:30	0	0	5	0	5		
07:45	1	0	3	0	4	15:45	0	0	4	0	4		
08:00	2	0	5	0	7	16:00	0	0	0	0	0		
08:15	0	0	6	0	6	16:15	1	0	4	0	5		
08:30	0	0	3	0	3	16:30	0	0	2	0	2		
08:45	1	0	4	0	5	16:45	1	0	3	0	4		
09:00	0	0	0	0	0	17:00	0	0	1	0	1		
09:15	0	0	4	0	4	17:15	1	0	0	0	1		
09:30	0	0	5	0	5	17:30	0	0	1	0	1		
09:45	2	0	1	0	3	17:45	0	0	1	0	1		
10:00	0	0	1	0	1	18:00	1	0	0	0	1		
10:15	0	0	3	0	3	18:15	0	0	1	0	1		
10:30	1	0	5	0	6	18:30	1	0	0	0	1		
10:45	1	0	4	0	5	18:45	0	0	2	0	2		
11:00	0	0	6	0	6	19:00	0	0	1	0	1		
11:15	1	0	7	0	8	19:15	1	0	1	0	2		
11:30	0	0	6	0	6	19:30	0	0	0	0	0		
11:45	3	0	3	0	6	19:45	0	0	1	0	1		
12:00	0	0	2	0	2	20:00	0	0	0	0	0		
12:15	1	0	6	0	7	20:15	0	0	2	0	2		
12:30	0	0	4	0	4	20:30	0	0	0	0	0		
12:45	0	0	1	0	1	20:45	0	0	0	0	0		
13:00	0	0	4	0	4	21:00	0	0	0	0	0		
13:15	1	0	2	0	3	21:15	0	0	0	0	0		
13:30	1	0	4	0	5	21:30	0	0	0	0	0		
13:45	1	0	5	0	6	21:45	0	0	0	0	0		
14:00	1	0	3	0	4	22:00	0	0	0	0	0		
SUBTOTAL					124	SUBTOTAL					53		
TOTAL GERAL					177								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	0	0	0	0	0	14:00-15:00	1	0	11	0	12
07:00-08:00	5	2	14	0	21	15:00-16:00	0	0	15	0	15
08:00-09:00	1	0	13	0	14	16:00-17:00	2	0	10	0	12
09:00-10:00	2	0	11	0	13	17:00-18:00	2	0	2	0	4
10:00-11:00	2	0	18	0	20	18:00-19:00	1	0	4	0	5
11:00-12:00	4	0	18	0	22	19:00-20:00	1	0	2	0	3
12:00-13:00	1	0	15	0	16	20:00-21:00	0	0	2	0	2
13:00-14:00	4	0	14	0	18	21:00-22:00	0	0	0	0	0
SUBTOTAL	19	2	103	0	124	SUBTOTAL	7	0	46	0	53
TOTAL GERAL	26	2	149	0	177						

GRÁFICOS



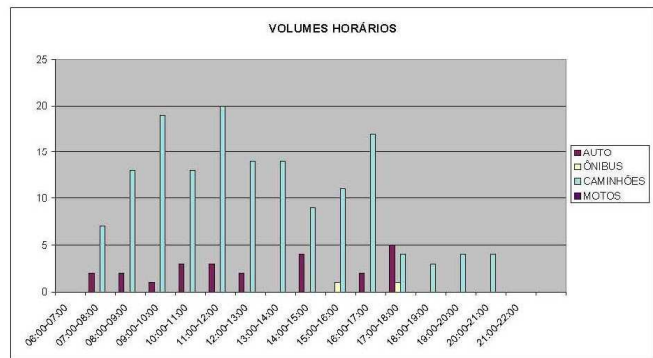
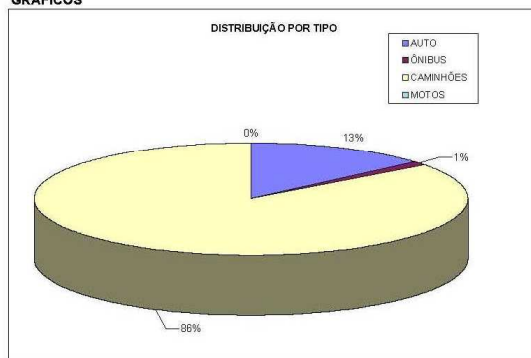
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA:		(via interna para Aterro Sanitário) RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 253			SENTIDO:		8 - RODOVIA			PONTO:		2
DATA:		27/6/2019			TEMPO		GAROA		TRÁFEGO:		NORMAL	
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	
06:15	0	0	0	0	0	14:15	2	0	3	0	5	
06:30	0	0	0	0	0	14:30	1	0	2	0	3	
06:45	0	0	0	0	0	14:45	0	0	2	0	2	
07:00	0	0	0	0	0	15:00	1	0	2	0	3	
07:15	0	0	5	0	5	15:15	0	0	1	0	1	
07:30	0	0	0	0	0	15:30	0	0	6	0	6	
07:45	1	0	1	0	2	15:45	0	0	2	0	2	
08:00	1	0	1	0	2	16:00	0	1	2	0	3	
08:15	2	0	7	0	9	16:15	0	0	6	0	6	
08:30	0	0	1	0	1	16:30	1	0	3	0	4	
08:45	0	0	2	0	2	16:45	1	0	5	0	6	
09:00	0	0	3	0	3	17:00	0	0	3	0	3	
09:15	0	0	6	0	6	17:15	3	1	3	0	7	
09:30	0	0	5	0	5	17:30	2	0	0	0	2	
09:45	0	0	6	0	6	17:45	0	0	0	0	0	
10:00	1	0	2	0	3	18:00	0	0	1	0	1	
10:15	1	0	0	0	1	18:15	0	0	1	0	1	
10:30	0	0	5	0	5	18:30	0	0	0	0	0	
10:45	1	0	4	0	5	18:45	0	0	1	0	1	
11:00	1	0	4	0	5	19:00	0	0	1	0	1	
11:15	1	0	4	0	5	19:15	0	0	2	0	2	
11:30	1	0	7	0	8	19:30	0	0	2	0	2	
11:45	0	0	4	0	4	19:45	0	0	0	0	0	
12:00	1	0	5	0	6	20:00	0	0	0	0	0	
12:15	0	0	1	0	1	20:15	0	0	1	0	1	
12:30	1	0	4	0	5	20:30	0	0	1	0	1	
12:45	1	0	5	0	6	20:45	0	0	0	0	0	
13:00	0	0	4	0	4	21:00	0	0	2	0	2	
13:15	0	0	5	0	5	21:15	0	0	0	0	0	
13:30	0	0	2	0	2	21:30	0	0	0	0	0	
13:45	0	0	3	0	3	21:45	0	0	0	0	0	
14:00	0	0	4	0	4	22:00	0	0	0	0	0	
SUBTOTAL					113	SUBTOTAL					65	
TOTAL GERAL					178							

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	0	0	0	0	0	14:00-15:00	4	0	9	0	13
07:00-08:00	2	0	7	0	9	15:00-16:00	0	1	11	0	12
08:00-09:00	2	0	13	0	15	16:00-17:00	2	0	17	0	19
09:00-10:00	1	0	19	0	20	17:00-18:00	5	1	4	0	10
10:00-11:00	3	0	13	0	16	18:00-19:00	0	0	3	0	3
11:00-12:00	3	0	20	0	23	19:00-20:00	0	0	4	0	4
12:00-13:00	2	0	14	0	16	20:00-21:00	0	0	4	0	4
13:00-14:00	0	0	14	0	14	21:00-22:00	0	0	0	0	0
SUBTOTAL	13	0	100	0	113	SUBTOTAL	11	2	52	0	65
TOTAL GERAL	24	2	152	0	178						

GRÁFICOS



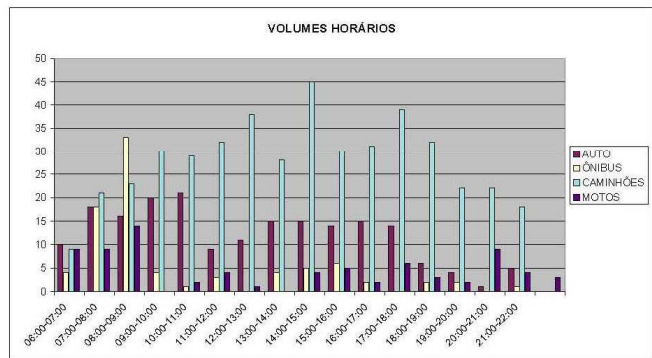
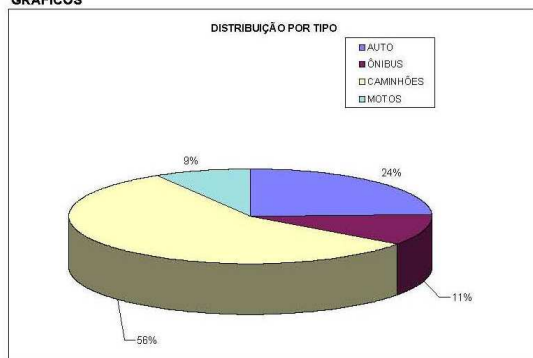
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 267,5						SENTIDO: 9 - RETORNO GUARUJÁ						PONTO: 3	
DATA: 27/6/2019						TEMPO		GAROA		TRÁFEGO: NORMAL			
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	2	1	2	1	6	14:15	2	0	10	1	13		
06:30	2	0	3	2	7	14:30	4	0	18	1	23		
06:45	2	2	2	2	8	14:45	5	1	10	2	18		
07:00	4	1	2	4	11	15:00	4	4	7	1	16		
07:15	5	2	1	6	14	15:15	1	1	3	0	5		
07:30	3	3	5	1	12	15:30	2	3	15	1	21		
07:45	5	7	5	5	22	15:45	6	2	7	0	15		
08:00	5	6	10	2	23	16:00	5	0	5	1	11		
08:15	4	13	4	0	21	16:15	3	0	11	3	17		
08:30	1	15	2	0	18	16:30	7	1	7	2	17		
08:45	9	1	7	0	17	16:45	2	0	8	1	11		
09:00	2	4	10	0	16	17:00	3	1	5	0	9		
09:15	7	3	6	1	17	17:15	7	0	9	0	16		
09:30	4	0	10	0	14	17:30	2	0	4	0	6		
09:45	6	1	8	1	16	17:45	3	0	17	1	21		
10:00	3	0	6	0	9	18:00	2	0	9	2	13		
10:15	4	1	7	3	15	18:15	2	0	6	0	8		
10:30	5	0	4	0	9	18:30	0	2	7	1	10		
10:45	2	0	9	0	11	18:45	1	0	11	0	12		
11:00	10	0	9	1	20	19:00	3	0	8	1	12		
11:15	2	2	2	0	6	19:15	1	2	6	3	12		
11:30	0	0	11	0	11	19:30	1	0	3	0	4		
11:45	2	0	7	0	9	19:45	0	0	8	1	9		
12:00	5	1	12	1	19	20:00	2	0	5	5	12		
12:15	3	0	17	0	20	20:15	0	0	7	1	8		
12:30	4	0	6	0	10	20:30	1	0	6	1	8		
12:45	1	0	10	0	11	20:45	0	0	4	0	4		
13:00	3	0	5	0	8	21:00	0	0	5	2	7		
13:15	5	1	8	1	15	21:15	1	0	0	0	1		
13:30	1	0	7	1	9	21:30	0	0	6	1	7		
13:45	4	2	4	0	10	21:45	3	0	7	1	11		
14:00	5	1	9	2	17	22:00	1	1	5	1	8		
SUBTOTAL					431	SUBTOTAL					365		
TOTAL GERAL					796								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	10	4	9	9	32	14:00-15:00	15	5	4,5	5	70
07:00-08:00	18	18	21	14	71	15:00-16:00	14	6	3,0	2	62
08:00-09:00	16	3,3	23	0	72	16:00-17:00	15	2	3,1	6	54
09:00-10:00	20	4	30	2	56	17:00-18:00	14	0	3,9	3	56
10:00-11:00	21	1	29	4	55	18:00-19:00	6	2	3,2	2	42
11:00-12:00	9	3	32	1	45	19:00-20:00	4	2	2,2	9	37
12:00-13:00	11	0	38	0	49	20:00-21:00	1	0	2,2	4	27
13:00-14:00	15	4	28	4	51	21:00-22:00	5	1	1,8	3	27
SUBTOTAL	120	67	210	34	431	SUBTOTAL	74	18	23,9	34	365
TOTAL GERAL	194	85	449	68	796						

GRÁFICOS



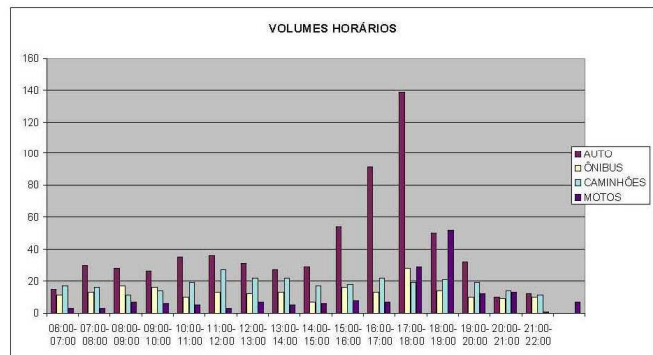
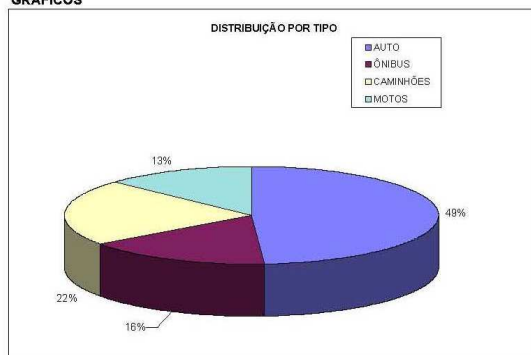
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 267,5					SENTIDO: 10 - SÃO PAULO - MARGINAL					PONTO: 3	
DATA: 27/6/2019					TEMPO	GAROA		TRAFEGO: NORMAL			
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:15	4	3	8	2	17	14:15	3	4	2	4	13
06:30	5	4	4	1	14	14:30	7	2	2	2	13
06:45	3	4	2	0	9	14:45	8	1	6	2	17
07:00	3	0	3	0	6	15:00	11	0	7	0	18
07:15	12	1	5	2	20	15:15	16	7	2	4	29
07:30	8	6	6	2	22	15:30	15	3	5	1	24
07:45	6	2	4	3	15	15:45	10	3	5	1	19
08:00	4	4	1	0	9	16:00	13	3	6	1	23
08:15	8	4	2	2	16	16:15	18	1	5	6	30
08:30	2	7	2	1	12	16:30	19	3	5	5	32
08:45	11	6	3	3	23	16:45	32	5	8	11	56
09:00	7	0	4	0	11	17:00	22	4	4	7	37
09:15	3	4	4	3	14	17:15	39	5	7	5	56
09:30	5	4	4	1	14	17:30	47	8	6	28	89
09:45	9	4	4	0	17	17:45	44	12	5	15	76
10:00	9	4	2	1	16	18:00	9	3	1	4	17
10:15	9	1	7	1	18	18:15	17	1	8	3	29
10:30	7	4	1	0	12	18:30	9	6	11	5	31
10:45	12	3	1	2	18	18:45	13	3	0	3	19
11:00	7	2	10	0	19	19:00	11	4	2	1	18
11:15	7	2	6	1	16	19:15	12	0	4	5	21
11:30	8	4	1	3	16	19:30	8	3	1	3	15
11:45	7	5	9	2	23	19:45	7	4	8	3	22
12:00	14	2	11	1	28	20:00	5	3	6	2	16
12:15	14	3	7	3	27	20:15	1	1	5	1	8
12:30	8	3	8	0	19	20:30	2	2	5	0	9
12:45	5	1	4	0	10	20:45	3	5	1	0	9
13:00	4	5	3	2	14	21:00	4	1	3	0	8
13:15	2	4	6	1	13	21:15	1	3	1	0	5
13:30	8	3	9	3	23	21:30	4	3	6	3	16
13:45	5	4	5	1	15	21:45	7	4	4	3	18
14:00	12	2	2	1	17	22:00	0	0	0	1	1
SUBTOTAL					523	SUBTOTAL					794
TOTAL GERAL					1317						

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	15	11	17	3	46	14:00-15:00	29	7	17	8	61
07:00-08:00	30	13	16	7	66	15:00-16:00	54	16	18	7	95
08:00-09:00	28	17	11	6	62	16:00-17:00	91	13	22	29	155
09:00-10:00	26	16	14	5	61	17:00-18:00	139	28	19	52	238
10:00-11:00	35	10	19	3	67	18:00-19:00	50	14	21	12	97
11:00-12:00	36	13	27	7	83	19:00-20:00	32	10	19	13	74
12:00-13:00	31	12	22	5	70	20:00-21:00	10	9	14	1	34
13:00-14:00	27	13	22	6	68	21:00-22:00	12	10	11	7	40
SUBTOTAL	228	105	148	42	523	SUBTOTAL	417	107	141	129	794
TOTAL GERAL	645	212	289	171	1317						

GRÁFICOS



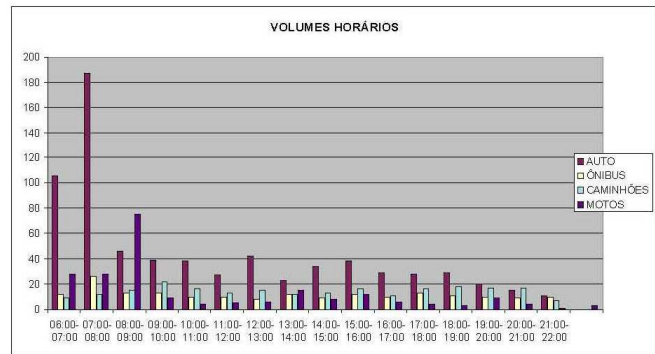
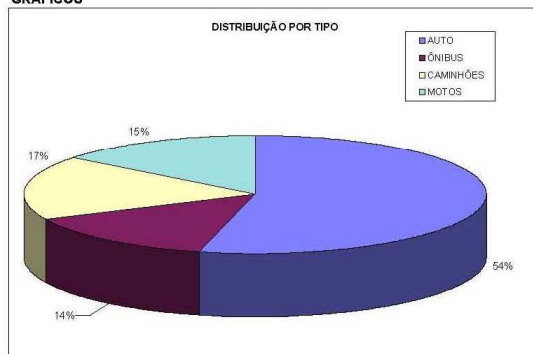
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA:		RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 267,5			SENTIDO:		11 - GUARUJÁ (MARGINAL)			PONTO:		3
DATA:		27/6/2019			TEMPO		GAROA			TRAFEGO:		NORMAL
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	
06:15	8	1	1	1	11	14:15	5	2	3	5	15	
06:30	14	1	1	1	17	14:30	9	0	2	2	13	
06:45	25	5	4	6	40	14:45	9	2	4	1	16	
07:00	58	5	3	20	86	15:00	11	5	4	4	24	
07:15	60	5	5	30	100	15:15	6	2	4	1	13	
07:30	46	5	3	27	81	15:30	8	2	2	0	12	
07:45	40	5	2	9	56	15:45	8	2	3	3	16	
08:00	41	11	2	9	63	16:00	16	6	7	2	31	
08:15	10	4	2	3	29	16:15	8	1	4	1	14	
08:30	11	2	6	3	22	16:30	5	3	2	1	11	
08:45	8	3	4	2	17	16:45	8	5	4	1	18	
09:00	8	4	3	1	16	17:00	8	1	1	1	11	
09:15	10	3	7	0	20	17:15	7	3	5	0	15	
09:30	10	7	3	1	21	17:30	7	6	5	1	19	
09:45	12	2	5	1	20	17:45	9	1	2	1	13	
10:00	7	1	7	2	17	18:00	5	3	4	1	13	
10:15	6	4	6	1	17	18:15	6	3	5	1	15	
10:30	12	1	4	1	18	18:30	5	4	4	4	17	
10:45	7	1	1	1	10	18:45	7	1	4	1	13	
11:00	13	4	5	2	24	19:00	11	3	5	3	22	
11:15	4	2	2	0	8	19:15	7	3	3	0	13	
11:30	9	3	1	3	16	19:30	5	2	1	1	9	
11:45	7	1	2	0	10	19:45	5	3	10	3	21	
12:00	7	4	8	3	22	20:00	3	2	3	0	8	
12:15	12	2	6	3	23	20:15	1	4	5	0	10	
12:30	10	2	4	2	18	20:30	7	1	6	1	15	
12:45	10	2	5	7	24	20:45	4	2	3	0	9	
13:00	10	2	0	3	15	21:00	3	2	3	0	8	
13:15	7	4	2	2	15	21:15	1	3	1	1	6	
13:30	5	2	2	1	10	21:30	4	2	2	1	9	
13:45	5	3	5	4	17	21:45	5	1	3	0	9	
14:00	6	3	3	1	13	22:00	1	4	1	1	7	
SUBTOTAL					875	SUBTOTAL					445	
TOTAL GERAL					1320							

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	105	12	9	28	154	14:00-15:00	34	9	13	12	68
07:00-08:00	187	26	12	75	300	15:00-16:00	38	12	16	6	72
08:00-09:00	46	13	15	9	83	16:00-17:00	29	10	11	4	54
09:00-10:00	39	13	22	4	78	17:00-18:00	28	13	16	3	60
10:00-11:00	38	10	16	5	69	18:00-19:00	29	11	18	9	67
11:00-12:00	27	10	13	6	56	19:00-20:00	20	10	17	4	51
12:00-13:00	42	8	15	15	80	20:00-21:00	15	9	17	1	42
13:00-14:00	23	12	12	8	55	21:00-22:00	11	10	7	3	31
SUBTOTAL	507	104	114	150	875	SUBTOTAL	204	84	115	42	446
TOTAL GERAL	711	188	229	192	1320						

GRÁFICOS



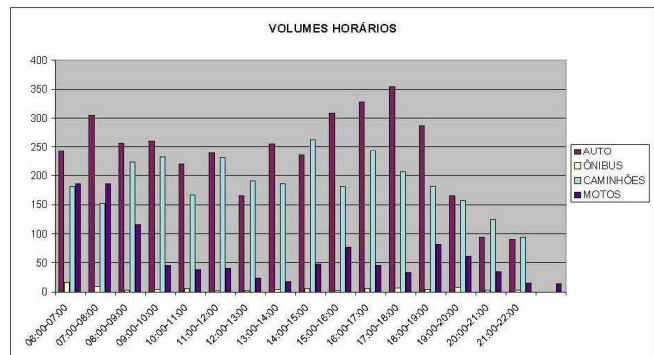
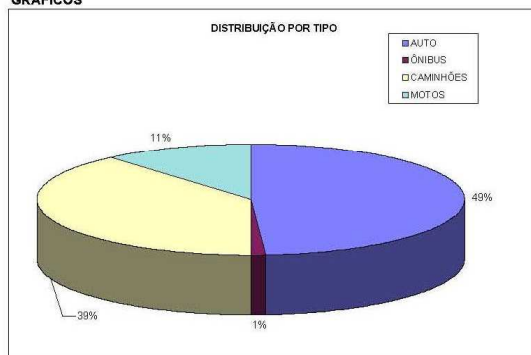
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248						SENTIDO: 1 - SÃO PAULO						PONTO: 1	
DATA: 25/6/2019						TEMPO: BOM		TRAFEGO: NORMAL					
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	49	5	62	38	154	14:15	64	3	66	27	160		
06:30	69	5	33	42	149	14:30	61	1	60	29	151		
06:45	67	3	48	58	176	14:45	55	1	65	13	134		
07:00	58	3	38	47	146	15:00	55	1	71	8	135		
07:15	96	2	38	32	168	15:15	82	0	57	4	143		
07:30	69	4	32	32	137	15:30	65	0	59	17	141		
07:45	76	3	39	32	150	15:45	68	1	46	8	123		
08:00	64	1	43	19	127	16:00	93	1	13	17	130		
08:15	57	0	43	11	111	16:15	86	2	62	4	154		
08:30	63	1	54	8	126	16:30	72	0	61	9	142		
08:45	67	0	73	13	153	16:45	68	2	64	12	146		
09:00	70	2	53	13	138	17:00	102	2	56	9	169		
09:15	54	1	66	7	128	17:15	83	1	33	12	129		
09:30	67	2	51	9	129	17:30	88	3	44	21	156		
09:45	67	0	52	14	133	17:45	78	3	47	31	159		
10:00	72	1	63	9	145	18:00	106	0	82	18	206		
10:15	66	2	55	16	139	18:15	79	1	44	21	145		
10:30	39	1	26	10	76	18:30	74	0	43	18	135		
10:45	62	0	38	8	108	18:45	79	3	38	15	135		
11:00	52	3	48	7	110	19:00	55	0	56	7	118		
11:15	58	0	66	4	128	19:15	34	2	57	16	109		
11:30	63	0	66	4	133	19:30	53	4	33	11	101		
11:45	66	1	47	12	126	19:45	49	2	39	5	95		
12:00	51	1	51	3	106	20:00	29	0	28	3	60		
12:15	27	1	41	0	69	20:15	38	1	42	6	87		
12:30	25	0	44	1	70	20:30	24	0	26	3	53		
12:45	65	0	66	8	139	20:45	17	0	28	5	50		
13:00	48	1	39	9	97	21:00	14	2	28	1	45		
13:15	55	2	53	7	117	21:15	16	2	24	3	45		
13:30	53	0	42	8	103	21:30	12	0	18	1	31		
13:45	72	0	52	15	139	21:45	9	1	26	4	40		
14:00	75	2	39	18	134	22:00	53	0	26	6	85		
SUBTOTAL					4064	SUBTOTAL					3712		
TOTAL GERAL					7776								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	243	16	181	185	625	14:00-15:00	235	6	262	77	580
07:00-08:00	305	10	152	115	582	15:00-16:00	308	2	181	46	537
08:00-09:00	257	3	223	45	528	16:00-17:00	328	6	243	34	611
09:00-10:00	260	4	232	39	535	17:00-18:00	355	7	206	82	650
10:00-11:00	219	6	167	41	433	18:00-19:00	287	4	181	61	533
11:00-12:00	238	2	230	23	493	19:00-20:00	165	8	157	35	365
12:00-13:00	165	2	190	18	375	20:00-21:00	93	3	124	15	235
13:00-14:00	255	4	186	48	493	21:00-22:00	90	3	94	14	201
SUBTOTAL	1942	47	1561	514	4064	SUBTOTAL	1861	39	1448	364	3712
TOTAL GERAL	3803	86	3009	878	7776						

GRÁFICOS



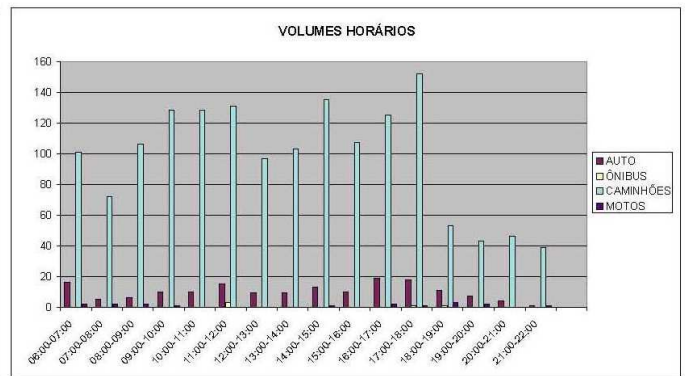
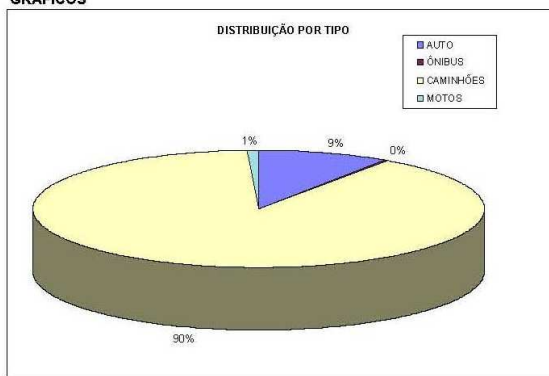
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248					SENTIDO: 2 - RETORNO PARA SÃO PAULO					PONTO: 1	
DATA: 25/8/2019					TEMPO	BOM		TRÁFEGO:		NORMAL	
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:15	8	0	26	0	34	14:15	1	0	37	0	38
06:30	2	0	25	2	29	14:30	5	0	47	0	52
06:45	4	0	28	0	32	14:45	5	0	32	0	37
07:00	2	0	22	0	24	15:00	2	0	19	0	21
07:15	2	0	15	1	18	15:15	2	0	22	0	24
07:30	2	0	15	0	17	15:30	0	0	27	0	27
07:45	1	0	22	0	23	15:45	5	0	33	1	39
08:00	0	0	20	1	21	16:00	3	0	25	1	29
08:15	0	0	18	0	18	16:15	4	0	24	0	28
08:30	1	0	28	1	30	16:30	3	0	28	0	31
08:45	1	0	22	0	23	16:45	5	0	31	0	36
09:00	4	0	38	0	42	17:00	7	0	42	1	50
09:15	1	0	25	0	26	17:15	4	0	39	0	43
09:30	4	0	21	0	25	17:30	5	1	66	0	72
09:45	2	0	32	0	34	17:45	2	0	21	0	23
10:00	3	0	50	0	53	18:00	7	0	26	3	36
10:15	0	0	45	0	45	18:15	7	0	16	1	24
10:30	3	0	35	0	38	18:30	2	1	17	1	21
10:45	5	0	26	0	31	18:45	1	0	12	0	13
11:00	2	0	22	0	24	19:00	1	0	8	0	9
11:15	2	1	21	0	24	19:15	2	0	16	0	18
11:30	0	2	8	0	10	19:30	2	0	7	0	9
11:45	9	0	78	0	85	19:45	1	0	16	0	17
12:00	4	0	26	0	30	20:00	2	0	4	0	6
12:15	5	0	28	0	33	20:15	1	0	7	0	8
12:30	3	0	22	0	25	20:30	0	0	8	0	8
12:45	0	0	24	0	24	20:45	2	0	17	0	19
13:00	1	0	23	0	24	21:00	1	0	14	1	16
13:15	3	0	26	0	29	21:15	1	0	13	0	14
13:30	1	0	30	0	31	21:30	0	0	10	0	10
13:45	4	0	17	1	22	21:45	0	0	10	0	10
14:00	1	0	30	0	31	22:00	0	0	6	0	6
SUBTOTAL					956	SUBTOTAL					794
TOTAL GERAL					1749						

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	18	0	101	2	119	14:00-15:00	13	0	135	0	148
07:00-08:00	5	0	72	2	79	15:00-16:00	10	0	107	2	119
08:00-09:00	6	0	106	1	113	16:00-17:00	19	0	125	1	145
09:00-10:00	10	0	128	0	138	17:00-18:00	18	1	152	3	174
10:00-11:00	10	0	128	0	138	18:00-19:00	11	1	53	2	67
11:00-12:00	15	3	131	0	149	19:00-20:00	7	0	43	0	50
12:00-13:00	9	0	97	0	106	20:00-21:00	4	0	48	1	51
13:00-14:00	9	0	103	1	113	21:00-22:00	1	0	39	0	40
SUBTOTAL	80	3	866	6	955	SUBTOTAL	83	2	700	8	794
TOTAL GERAL	163	5	1566	15	1749						

GRÁFICOS



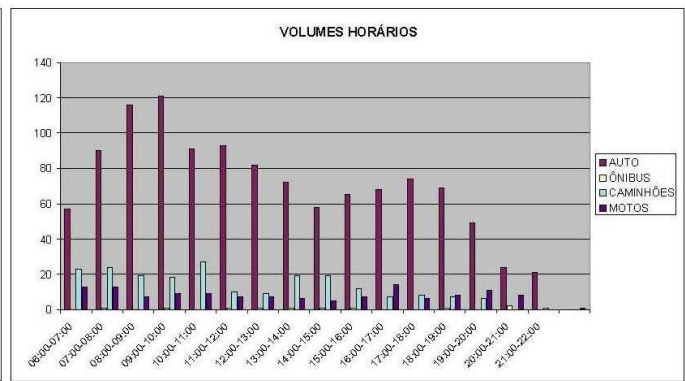
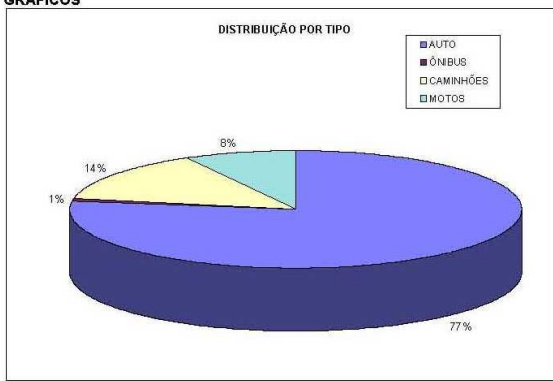
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248						SENTIDO: 3 - BERTIOGA /SÃO SEBASTIÃO					PONTO: 1
DATA: 25/8/2019						TEMPO: BOM		TRÁFEGO: NORMAL			
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:15	8	0	3	1	10	14:15	14	0	7	2	23
06:30	22	0	4	2	28	14:30	11	0	4	2	17
06:45	19	0	10	4	33	14:45	16	0	4	1	21
07:00	10	0	6	6	22	15:00	17	1	4	2	24
07:15	21	0	5	4	30	15:15	20	0	4	1	25
07:30	19	0	6	1	26	15:30	12	1	4	10	27
07:45	23	1	11	0	35	15:45	19	0	3	2	24
08:00	27	0	2	2	31	16:00	14	0	1	1	16
08:15	22	0	5	2	29	16:15	18	0	0	2	20
08:30	23	0	5	0	28	16:30	17	0	3	2	22
08:45	37	0	2	3	42	16:45	16	0	3	2	21
09:00	34	0	7	4	45	17:00	17	0	1	0	18
09:15	25	0	2	1	28	17:15	13	0	1	1	15
09:30	40	0	4	3	47	17:30	17	0	4	1	22
09:45	18	0	9	2	29	17:45	25	0	1	4	30
10:00	38	1	3	3	45	18:00	19	0	2	2	23
10:15	19	0	9	0	28	18:15	25	0	3	2	30
10:30	26	0	9	3	38	18:30	10	0	2	2	14
10:45	24	0	4	2	30	18:45	20	1	1	4	26
11:00	22	0	5	2	29	19:00	14	0	1	3	18
11:15	28	0	1	0	29	19:15	13	0	0	2	15
11:30	23	0	3	0	26	19:30	13	0	1	0	14
11:45	20	0	4	4	28	19:45	10	0	3	1	14
12:00	22	1	2	3	28	20:00	13	0	2	5	20
12:15	23	0	2	1	26	20:15	5	2	0	0	7
12:30	16	1	1	2	20	20:30	9	0	0	0	9
12:45	20	0	4	2	26	20:45	9	0	0	0	9
13:00	23	0	2	1	26	21:00	1	0	0	0	1
13:15	20	0	9	2	31	21:15	4	0	1	0	5
13:30	28	1	5	1	35	21:30	5	0	0	1	6
13:45	6	0	3	1	10	21:45	3	0	0	0	3
14:00	18	0	2	1	21	22:00	9	0	0	0	9
SUBTOTAL					939	SUBTOTAL					548
TOTAL GERAL					1487						

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	57	0	23	13	93	14:00-15:00	58	1	19	7	85
07:00-08:00	90	1	24	7	122	15:00-16:00	65	1	12	14	92
08:00-09:00	116	0	19	9	144	16:00-17:00	68	0	7	6	81
09:00-10:00	121	1	18	9	149	17:00-18:00	74	0	8	8	90
10:00-11:00	91	0	27	7	125	18:00-19:00	69	1	7	11	88
11:00-12:00	93	1	10	7	111	19:00-20:00	49	0	6	8	63
12:00-13:00	82	1	9	6	98	20:00-21:00	24	2	0	0	26
13:00-14:00	72	1	19	5	97	21:00-22:00	21	0	1	1	23
SUBTOTAL	722	5	149	83	939	SUBTOTAL	428	5	80	55	548
TOTAL GERAL	1150	10	209	118	1487						

GRÁFICOS



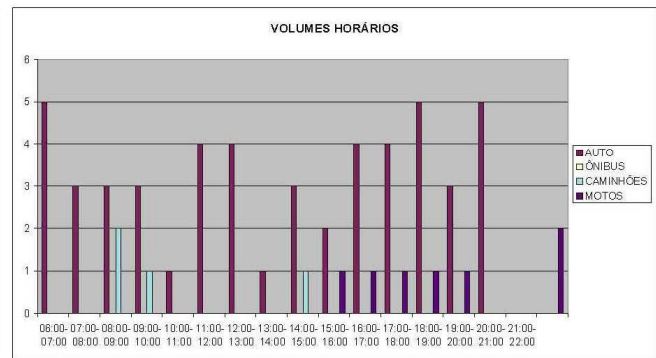
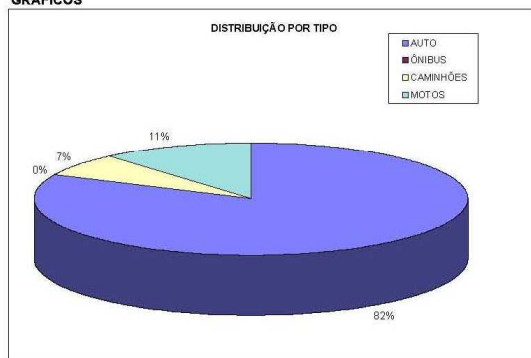
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA:		RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248				SENTIDO:		4- RETORNO PARA GUARUJÁ				PONTO:		1		
DATA:		25/6/2019				TEMPO:		BOM				TRAFEGO:		NORMAL		
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL					
06:15	1	0	0	0	1	14:15	1	0	0	0	1					
06:30	3	0	0	0	3	14:30	1	0	0	0	1					
06:45	0	0	0	0	0	14:45	1	0	0	0	1					
07:00	1	0	0	0	1	15:00	0	0	1	0	1					
07:15	1	0	0	0	1	15:15	0	0	0	0	0					
07:30	0	0	0	0	0	15:30	2	0	0	0	2					
07:45	0	0	0	0	0	15:45	0	0	0	0	1					
08:00	2	0	0	0	2	16:00	0	0	0	0	0					
08:15	0	0	0	0	0	16:15	1	0	0	0	1					
08:30	0	0	0	0	0	16:30	1	0	0	0	1					
08:45	2	0	1	0	3	16:45	1	0	0	0	1					
09:00	1	0	1	0	2	17:00	1	0	0	0	1					
09:15	0	0	1	0	1	17:15	0	0	0	0	0					
09:30	1	0	0	0	1	17:30	2	0	0	0	1					
09:45	1	0	0	0	1	17:45	0	0	0	0	0					
10:00	1	0	0	0	1	18:00	2	0	0	0	2					
10:15	1	0	0	0	1	18:15	2	0	0	0	2					
10:30	0	0	0	0	0	18:30	1	0	0	0	1					
10:45	0	0	0	0	0	18:45	0	0	0	0	0					
11:00	0	0	0	0	0	19:00	2	0	0	0	2					
11:15	1	0	0	0	1	19:15	2	0	0	0	2					
11:30	1	0	0	0	1	19:30	0	0	0	0	0					
11:45	2	0	0	0	2	19:45	1	0	0	0	1					
12:00	0	0	0	0	0	20:00	0	0	0	0	0					
12:15	2	0	0	0	2	20:15	2	0	0	0	2					
12:30	0	0	0	0	0	20:30	0	0	0	0	0					
12:45	0	0	0	0	0	20:45	2	0	0	0	2					
13:00	2	0	0	0	2	21:00	1	0	0	0	1					
13:15	0	0	0	0	0	21:15	0	0	0	0	0					
13:30	1	0	0	0	1	21:30	0	0	0	0	1					
13:45	0	0	0	0	0	21:45	0	0	0	0	1					
14:00	0	0	0	0	0	22:00	0	0	0	0	0					
SUBTOTAL					27	SUBTOTAL					34					
TOTAL GERAL					61											

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	5	0	0	0	5	14:00-15:00	3	0	1	1	5
07:00-08:00	3	0	0	0	3	15:00-16:00	2	0	0	1	3
08:00-09:00	3	0	2	0	5	16:00-17:00	4	0	0	1	5
09:00-10:00	3	0	1	0	4	17:00-18:00	4	0	0	1	5
10:00-11:00	1	0	0	0	1	18:00-19:00	5	0	0	1	6
11:00-12:00	4	0	0	0	4	19:00-20:00	3	0	0	0	3
12:00-13:00	4	0	0	0	4	20:00-21:00	5	0	0	0	5
13:00-14:00	1	0	0	0	1	21:00-22:00	0	0	0	2	2
SUBTOTAL	24	0	3	0	27	SUBTOTAL	26	0	1	7	34
TOTAL GERAL	50	0	4	7	61						

GRÁFICOS



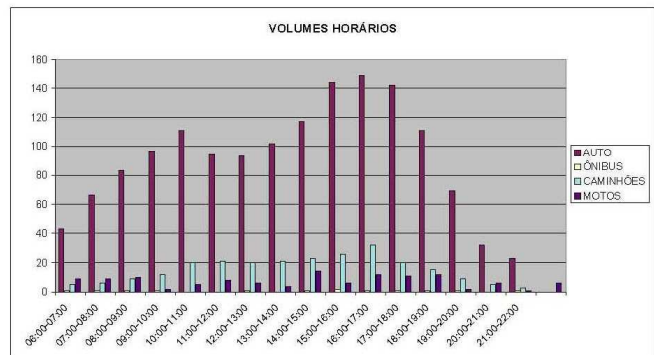
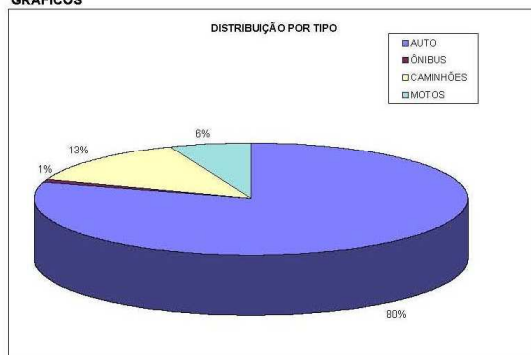
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248						SENTIDO: 5- VEM DE BERTIOGA SENTIDO SÃO PAULO						PONTO: 1	
DATA: 25/6/2019						TEMPO: BOM						TRAFEGO: NORMAL	
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	7	0	1	2	10	14:15	33	0	4	2	39		
06:30	9	1	3	3	16	14:30	20	1	8	2	31		
06:45	10	0	0	1	11	14:45	38	0	5	2	45		
07:00	17	0	1	3	21	15:00	26	0	6	0	32		
07:15	18	0	2	2	22	15:15	44	2	8	3	57		
07:30	16	0	1	3	20	15:30	28	0	5	1	34		
07:45	16	1	0	2	19	15:45	43	0	4	4	51		
08:00	16	0	3	3	22	16:00	29	0	9	4	42		
08:15	12	0	5	0	17	16:15	34	0	4	4	42		
08:30	26	0	0	1	27	16:30	52	0	11	1	64		
08:45	20	1	2	0	23	16:45	34	1	11	5	51		
09:00	25	0	2	1	28	17:00	29	0	6	1	36		
09:15	29	0	1	1	31	17:15	42	1	7	4	54		
09:30	28	0	4	0	32	17:30	36	0	4	2	42		
09:45	20	1	1	3	25	17:45	33	0	4	2	39		
10:00	19	0	6	1	26	18:00	31	0	5	4	40		
10:15	42	0	13	5	60	18:15	29	1	2	-2	30		
10:30	9	0	4	0	13	18:30	24	0	1	0	25		
10:45	33	0	1	1	35	18:45	34	0	7	2	43		
11:00	27	0	2	2	31	19:00	24	0	5	2	31		
11:15	26	0	6	2	34	19:15	22	1	2	2	27		
11:30	32	0	5	2	39	19:30	14	0	1	1	16		
11:45	14	0	5	1	20	19:45	22	0	3	1	26		
12:00	22	0	5	1	28	20:00	11	0	3	2	16		
12:15	26	1	6	2	35	20:15	4	0	1	1	6		
12:30	11	0	6	0	17	20:30	11	0	2	0	13		
12:45	33	0	6	1	40	20:45	8	0	2	0	10		
13:00	23	0	2	1	26	21:00	9	0	0	0	9		
13:15	18	0	5	1	24	21:15	8	0	0	1	9		
13:30	18	0	5	3	26	21:30	8	0	2	1	11		
13:45	35	0	7	5	47	21:45	2	1	0	1	4		
14:00	31	0	4	5	40	22:00	5	0	1	3	9		
SUBTOTAL					865	SUBTOTAL					984		
TOTAL GERAL					1849								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	43	1	5	9	58	14:00-15:00	117	1	23	6	147
07:00-08:00	66	1	6	10	83	15:00-16:00	144	2	26	12	184
08:00-09:00	83	1	9	2	95	16:00-17:00	149	1	32	11	193
09:00-10:00	96	1	12	5	114	17:00-18:00	142	1	20	12	175
10:00-11:00	111	0	20	8	139	18:00-19:00	111	1	15	2	129
11:00-12:00	94	0	21	6	121	19:00-20:00	69	1	9	6	85
12:00-13:00	93	1	20	4	118	20:00-21:00	32	0	5	1	38
13:00-14:00	102	0	21	14	137	21:00-22:00	23	1	3	6	33
SUBTOTAL	688	5	114	58	865	SUBTOTAL	787	8	133	56	994
TOTAL GERAL	1475	13	247	114	1849						

GRÁFICOS



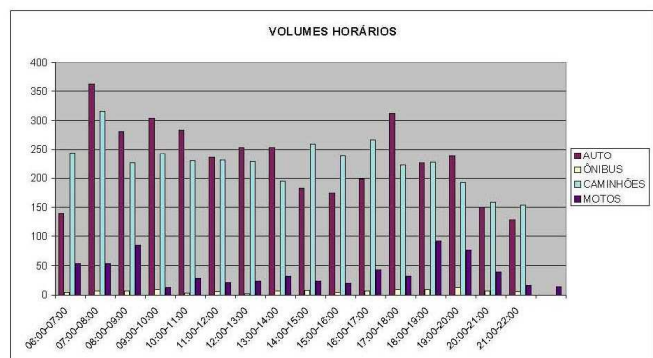
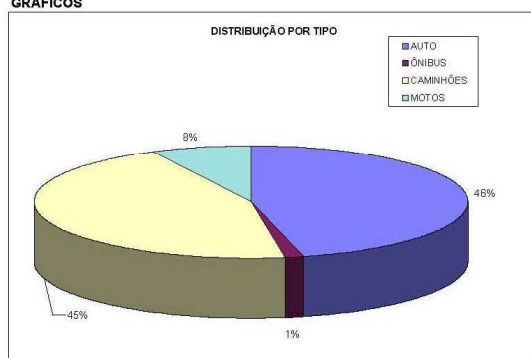
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 248						SENTIDO: 6 - GUARUJÁ						PONTO: 1	
DATA: 25/6/2019						TEMPO BOM						TRÁFEGO: NORMAL	
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	26	1	88	2	117	14:15	39	4	65	4	112		
06:30	33	2	63	10	108	14:30	40	1	85	1	127		
06:45	33	1	42	15	91	14:45	40	1	63	5	109		
07:00	47	1	51	27	126	15:00	64	2	47	10	123		
07:15	61	0	66	14	141	15:15	41	0	73	4	118		
07:30	92	2	76	12	182	15:30	45	0	54	11	110		
07:45	95	1	67	41	204	15:45	50	2	57	13	122		
08:00	115	4	107	18	244	16:00	38	3	53	15	109		
08:15	58	4	34	2	98	16:15	47	1	52	3	103		
08:30	65	1	62	6	134	16:30	22	1	52	11	86		
08:45	62	1	66	3	132	16:45	70	3	81	10	164		
09:00	96	1	64	2	163	17:00	59	2	81	8	150		
09:15	78	3	98	2	181	17:15	45	1	70	39	165		
09:30	70	2	6	11	89	17:30	83	3	38	15	139		
09:45	75	3	72	2	152	17:45	75	4	52	21	152		
10:00	81	2	68	13	162	18:00	109	1	62	17	189		
10:15	67	0	46	4	117	18:15	32	1	59	16	108		
10:30	86	1	63	2	152	18:30	69	2	51	21	143		
10:45	58	1	55	6	120	18:45	63	3	59	18	143		
11:00	72	1	65	9	147	19:00	61	3	58	22	144		
11:15	63	0	53	8	124	19:15	50	0	70	12	132		
11:30	50	2	78	4	134	19:30	71	8	56	7	142		
11:45	70	3	51	8	132	19:45	64	1	32	14	111		
12:00	52	1	48	4	105	20:00	53	4	34	6	97		
12:15	65	0	49	5	119	20:15	30	4	37	1	72		
12:30	56	0	53	7	116	20:30	47	1	52	5	105		
12:45	59	1	61	9	130	20:45	30	0	47	4	81		
13:00	73	1	65	11	150	21:00	43	2	23	6	74		
13:15	61	0	42	2	105	21:15	30	1	35	10	76		
13:30	61	1	41	7	110	21:30	31	0	41	2	74		
13:45	55	3	46	15	119	21:45	38	2	44	0	84		
14:00	76	3	66	0	145	22:00	29	3	34	2	68		
SUBTOTAL					4349	SUBTOTAL					3722		
TOTAL GERAL					8071								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	139	5	244	54	442	14:00-15:00	183	8	260	20	471
07:00-08:00	383	7	316	85	771	15:00-16:00	174	5	237	43	459
08:00-09:00	281	7	226	13	527	16:00-17:00	198	7	266	32	503
09:00-10:00	304	10	242	28	584	17:00-18:00	312	9	222	92	635
10:00-11:00	283	3	229	21	536	18:00-19:00	225	9	227	77	538
11:00-12:00	235	6	230	24	495	19:00-20:00	238	13	192	39	482
12:00-13:00	253	2	228	32	515	20:00-21:00	150	7	159	16	332
13:00-14:00	253	7	195	24	479	21:00-22:00	128	6	154	14	302
SUBTOTAL	2111	47	1910	281	4349	SUBTOTAL	1608	64	1717	333	3722
TOTAL GERAL	3719	111	3627	614	8071						

GRÁFICOS



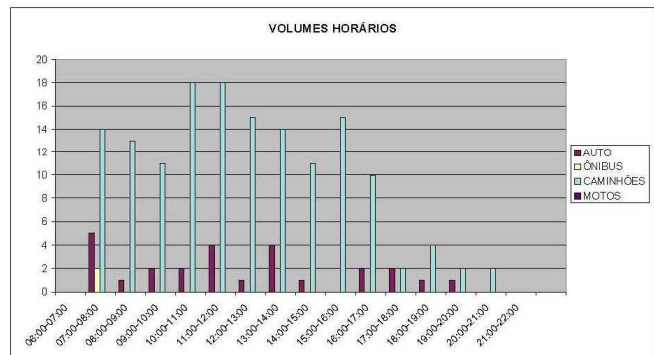
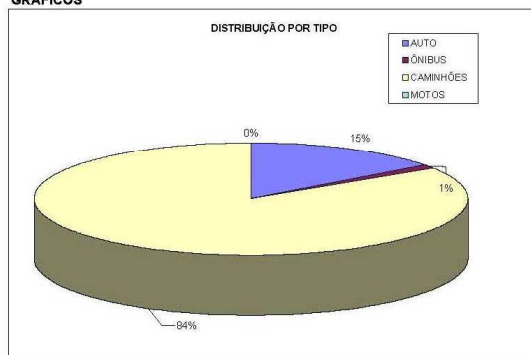
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA:		(via interna para Aterro Sanitário) RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 253			SENTIDO:		7 - ATERRRO			PONTO:		2
DATA:		27/6/2019			TEMPO		GAROA			TRÁFEGO:		NORMAL
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	
06:15	0	0	0	0	0	14:15	0	0	0	0	0	
06:30	0	0	0	0	0	14:30	0	0	5	0	5	
06:45	0	0	0	0	0	14:45	0	0	1	0	1	
07:00	0	0	0	0	0	15:00	1	0	5	0	6	
07:15	2	2	5	0	9	15:15	0	0	6	0	6	
07:30	0	0	1	0	1	15:30	0	0	5	0	5	
07:45	1	0	3	0	4	15:45	0	0	4	0	4	
08:00	2	0	5	0	7	16:00	0	0	0	0	0	
08:15	0	0	6	0	6	16:15	1	0	4	0	5	
08:30	0	0	3	0	3	16:30	0	0	2	0	2	
08:45	1	0	4	0	5	16:45	1	0	3	0	4	
09:00	0	0	0	0	0	17:00	0	0	1	0	1	
09:15	0	0	4	0	4	17:15	1	0	0	0	1	
09:30	0	0	5	0	5	17:30	0	0	1	0	1	
09:45	2	0	1	0	3	17:45	0	0	1	0	1	
10:00	0	0	1	0	1	18:00	1	0	0	0	1	
10:15	0	0	3	0	3	18:15	0	0	1	0	1	
10:30	1	0	5	0	6	18:30	1	0	0	0	1	
10:45	1	0	4	0	5	18:45	0	0	2	0	2	
11:00	0	0	6	0	6	19:00	0	0	1	0	1	
11:15	1	0	7	0	8	19:15	1	0	1	0	2	
11:30	0	0	6	0	6	19:30	0	0	0	0	0	
11:45	3	0	3	0	6	19:45	0	0	1	0	1	
12:00	0	0	2	0	2	20:00	0	0	0	0	0	
12:15	1	0	6	0	7	20:15	0	0	2	0	2	
12:30	0	0	4	0	4	20:30	0	0	0	0	0	
12:45	0	0	1	0	1	20:45	0	0	0	0	0	
13:00	0	0	4	0	4	21:00	0	0	0	0	0	
13:15	1	0	2	0	3	21:15	0	0	0	0	0	
13:30	1	0	4	0	5	21:30	0	0	0	0	0	
13:45	1	0	5	0	6	21:45	0	0	0	0	0	
14:00	1	0	3	0	4	22:00	0	0	0	0	0	
SUBTOTAL					124	SUBTOTAL					53	
TOTAL GERAL					177							

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	0	0	0	0	0	14:00-15:00	1	0	11	0	12
07:00-08:00	5	2	14	0	21	15:00-16:00	0	0	15	0	15
08:00-09:00	1	0	13	0	14	16:00-17:00	2	0	10	0	12
09:00-10:00	2	0	11	0	13	17:00-18:00	2	0	2	0	4
10:00-11:00	2	0	18	0	20	18:00-19:00	1	0	4	0	5
11:00-12:00	4	0	18	0	22	19:00-20:00	1	0	2	0	3
12:00-13:00	1	0	15	0	16	20:00-21:00	0	0	2	0	2
13:00-14:00	4	0	14	0	18	21:00-22:00	0	0	0	0	0
SUBTOTAL	19	2	103	0	124	SUBTOTAL	7	0	46	0	53
TOTAL GERAL	26	2	149	0	177						

GRÁFICOS



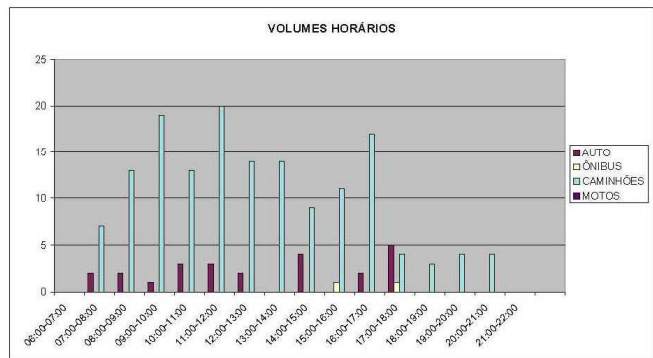
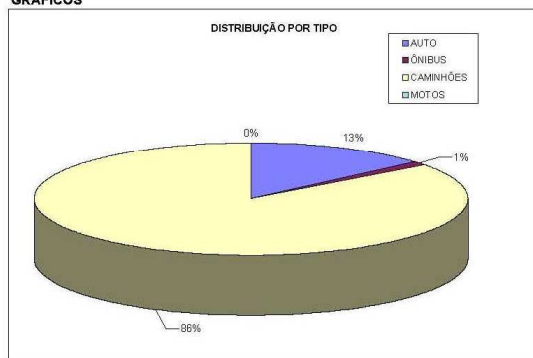
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: (via interna para Aterro Sanitário) RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 253						SENTIDO: 8 - RODOVIA						PONTO: 2	
DATA: 27/6/2019						TEMPO		GAROA		TRÁFEGO: NORMAL			
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	0	0	0	0	0	14:15	2	0	3	0	5		
06:30	0	0	0	0	0	14:30	1	0	2	0	3		
06:45	0	0	0	0	0	14:45	0	0	2	0	2		
07:00	0	0	0	0	0	15:00	1	0	2	0	3		
07:15	0	0	5	0	5	15:15	0	0	1	0	1		
07:30	0	0	0	0	0	15:30	0	0	6	0	6		
07:45	1	0	1	0	2	15:45	0	0	2	0	2		
08:00	1	0	1	0	2	16:00	0	1	2	0	3		
08:15	2	0	7	0	9	16:15	0	0	6	0	6		
08:30	0	0	1	0	1	16:30	1	0	3	0	4		
08:45	0	0	2	0	2	16:45	1	0	5	0	6		
09:00	0	0	3	0	3	17:00	0	0	3	0	3		
09:15	0	0	6	0	6	17:15	3	1	3	0	7		
09:30	0	0	5	0	5	17:30	2	0	0	0	2		
09:45	0	0	6	0	6	17:45	0	0	0	0	0		
10:00	1	0	2	0	3	18:00	0	0	1	0	1		
10:15	1	0	0	0	1	18:15	0	0	1	0	1		
10:30	0	0	5	0	5	18:30	0	0	0	0	0		
10:45	1	0	4	0	5	18:45	0	0	1	0	1		
11:00	1	0	4	0	5	19:00	0	0	1	0	1		
11:15	1	0	4	0	5	19:15	0	0	2	0	2		
11:30	1	0	7	0	8	19:30	0	0	2	0	2		
11:45	0	0	4	0	4	19:45	0	0	0	0	0		
12:00	1	0	5	0	6	20:00	0	0	0	0	0		
12:15	0	0	1	0	1	20:15	0	0	1	0	1		
12:30	1	0	4	0	5	20:30	0	0	1	0	1		
12:45	1	0	5	0	6	20:45	0	0	0	0	0		
13:00	0	0	4	0	4	21:00	0	0	2	0	2		
13:15	0	0	5	0	5	21:15	0	0	0	0	0		
13:30	0	0	2	0	2	21:30	0	0	0	0	0		
13:45	0	0	3	0	3	21:45	0	0	0	0	0		
14:00	0	0	4	0	4	22:00	0	0	0	0	0		
SUBTOTAL					113	SUBTOTAL					65		
TOTAL GERAL					178								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	0	0	0	0	0	14:00-15:00	4	0	9	0	13
07:00-08:00	2	0	7	0	9	15:00-16:00	0	1	11	0	12
08:00-09:00	2	0	13	0	15	16:00-17:00	2	0	17	0	19
09:00-10:00	1	0	19	0	20	17:00-18:00	5	1	4	0	10
10:00-11:00	3	0	13	0	16	18:00-19:00	0	0	3	0	3
11:00-12:00	3	0	20	0	23	19:00-20:00	0	0	4	0	4
12:00-13:00	2	0	14	0	16	20:00-21:00	0	0	4	0	4
13:00-14:00	0	0	14	0	14	21:00-22:00	0	0	0	0	0
SUBTOTAL	13	0	100	0	113	SUBTOTAL	11	2	52	0	65
TOTAL GERAL	24	2	152	0	178						

GRÁFICOS



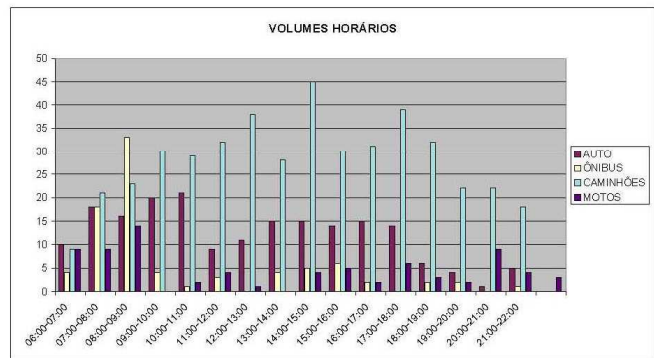
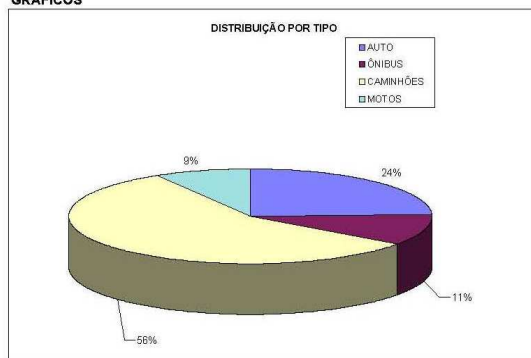
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 267,5						SENTIDO: 9 - RETORNO GUARUJÁ						PONTO: 3	
DATA: 27/6/2019						TEMPO		GAROA		TRÁFEGO: NORMAL			
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	2	1	2	1	6	14:15	2	0	10	1	13		
06:30	2	0	3	2	7	14:30	4	0	18	1	23		
06:45	2	2	2	2	8	14:45	5	1	10	2	18		
07:00	4	1	2	4	11	15:00	4	4	7	1	16		
07:15	5	2	1	6	14	15:15	1	1	3	0	5		
07:30	3	3	5	1	12	15:30	2	3	15	1	21		
07:45	5	7	5	5	22	15:45	6	2	7	0	15		
08:00	5	6	10	2	23	16:00	5	0	5	1	11		
08:15	4	13	4	0	21	16:15	3	0	11	3	17		
08:30	1	15	2	0	18	16:30	7	1	7	2	17		
08:45	9	1	7	0	17	16:45	2	0	8	1	11		
09:00	2	4	10	0	16	17:00	3	1	5	0	9		
09:15	7	3	6	1	17	17:15	7	0	9	0	16		
09:30	4	0	10	0	14	17:30	2	0	4	0	6		
09:45	6	1	8	1	16	17:45	3	0	17	1	21		
10:00	3	0	6	0	9	18:00	2	0	9	2	13		
10:15	4	1	7	3	15	18:15	2	0	6	0	8		
10:30	5	0	4	0	9	18:30	0	2	7	1	10		
10:45	2	0	9	0	11	18:45	1	0	11	0	12		
11:00	10	0	9	1	20	19:00	3	0	8	1	12		
11:15	2	2	2	0	6	19:15	1	2	6	3	12		
11:30	0	0	11	0	11	19:30	1	0	3	0	4		
11:45	2	0	7	0	9	19:45	0	0	8	1	9		
12:00	5	1	12	1	19	20:00	2	0	5	5	12		
12:15	3	0	17	0	20	20:15	0	0	7	1	8		
12:30	4	0	6	0	10	20:30	1	0	6	1	8		
12:45	1	0	10	0	11	20:45	0	0	4	0	4		
13:00	3	0	5	0	8	21:00	0	0	5	2	7		
13:15	5	1	8	1	15	21:15	1	0	0	0	1		
13:30	1	0	7	1	9	21:30	0	0	6	1	7		
13:45	4	2	4	0	10	21:45	3	0	7	1	11		
14:00	5	1	9	2	17	22:00	1	1	5	1	8		
SUBTOTAL					431	SUBTOTAL					365		
TOTAL GERAL					796								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	10	4	9	9	32	14:00-15:00	15	5	4,5	5	70
07:00-08:00	18	18	21	14	71	15:00-16:00	14	6	3,0	2	62
08:00-09:00	16	3,3	23	0	72	16:00-17:00	15	2	3,1	6	54
09:00-10:00	20	4	30	2	56	17:00-18:00	14	0	3,9	3	56
10:00-11:00	21	1	29	4	55	18:00-19:00	6	2	3,2	2	42
11:00-12:00	9	3	32	1	45	19:00-20:00	4	2	2,2	9	37
12:00-13:00	11	0	38	0	49	20:00-21:00	1	0	2,2	4	27
13:00-14:00	15	4	28	4	51	21:00-22:00	5	1	1,8	3	27
SUBTOTAL	120	67	210	34	431	SUBTOTAL	74	18	23,9	34	365
TOTAL GERAL	194	85	449	68	796						

GRÁFICOS



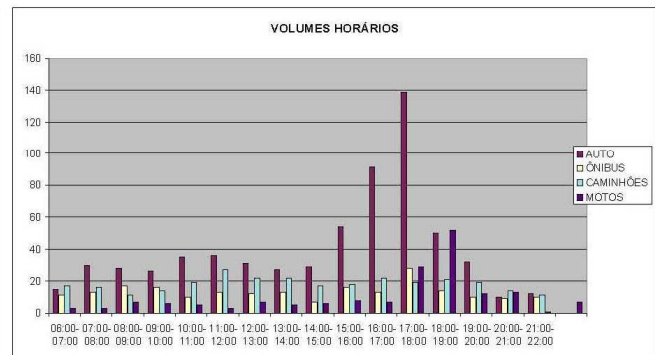
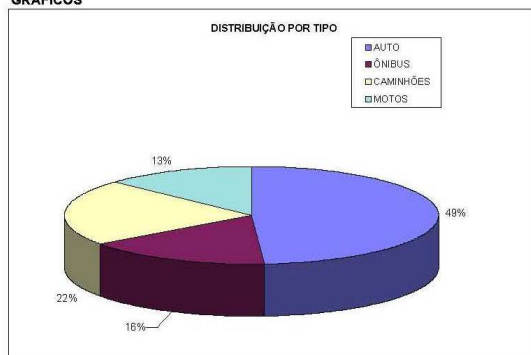
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA:		RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 267,5				SENTIDO:		10 - SÃO PAULO - MARGINAL				PONTO:		3		
DATA:		27/6/2019				TEMPO		GAROA				TRAFEGO:		NORMAL		
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL					
06:15	4	3	8	2	17	14:15	3	4	2	4	13					
06:30	5	4	4	1	14	14:30	7	2	2	2	13					
06:45	3	4	2	0	9	14:45	8	1	6	2	17					
07:00	3	0	3	0	6	15:00	11	0	7	0	18					
07:15	12	1	5	2	20	15:15	16	7	2	4	29					
07:30	8	6	6	2	22	15:30	15	3	5	1	24					
07:45	6	2	4	3	15	15:45	10	3	5	1	19					
08:00	4	4	1	0	9	16:00	13	3	6	1	23					
08:15	8	4	2	2	16	16:15	18	1	5	6	30					
08:30	2	7	2	1	12	16:30	19	3	5	5	32					
08:45	11	6	3	3	23	16:45	32	5	8	11	56					
09:00	7	0	4	0	11	17:00	22	4	4	7	37					
09:15	3	4	4	3	14	17:15	39	5	7	5	56					
09:30	5	4	4	1	14	17:30	47	8	6	28	89					
09:45	9	4	4	0	17	17:45	44	12	5	15	76					
10:00	9	4	2	1	16	18:00	9	3	1	4	17					
10:15	9	1	7	1	18	18:15	17	1	8	3	29					
10:30	7	4	1	0	12	18:30	9	6	11	5	31					
10:45	12	3	1	2	18	18:45	13	3	0	3	19					
11:00	7	2	10	0	19	19:00	11	4	2	1	18					
11:15	7	2	6	1	16	19:15	12	0	4	5	21					
11:30	8	4	1	3	16	19:30	8	3	1	3	15					
11:45	7	5	9	2	23	19:45	7	4	8	3	22					
12:00	14	2	11	1	28	20:00	5	3	6	2	16					
12:15	14	3	7	3	27	20:15	1	1	5	1	8					
12:30	8	3	8	0	19	20:30	2	2	5	0	9					
12:45	5	1	4	0	10	20:45	3	5	1	0	9					
13:00	4	5	3	2	14	21:00	4	1	3	0	8					
13:15	2	4	6	1	13	21:15	1	3	1	0	5					
13:30	8	3	9	3	23	21:30	4	3	6	3	16					
13:45	5	4	5	1	15	21:45	7	4	4	3	18					
14:00	12	2	2	1	17	22:00	0	0	0	1	1					
SUBTOTAL					523	SUBTOTAL					794					
TOTAL GERAL					1317											

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	15	11	17	3	46	14:00-15:00	29	7	17	8	61
07:00-08:00	30	13	16	7	66	15:00-16:00	54	16	18	7	95
08:00-09:00	28	17	11	6	62	16:00-17:00	91	13	22	29	155
09:00-10:00	26	16	14	5	61	17:00-18:00	139	28	19	52	238
10:00-11:00	35	10	19	3	67	18:00-19:00	50	14	21	12	97
11:00-12:00	36	13	27	7	83	19:00-20:00	32	10	19	13	74
12:00-13:00	31	12	22	5	70	20:00-21:00	10	9	14	1	34
13:00-14:00	27	13	22	6	68	21:00-22:00	12	10	11	7	40
SUBTOTAL	228	105	148	42	523	SUBTOTAL	417	107	141	129	794
TOTAL GERAL	645	212	289	171	1317						

GRÁFICOS



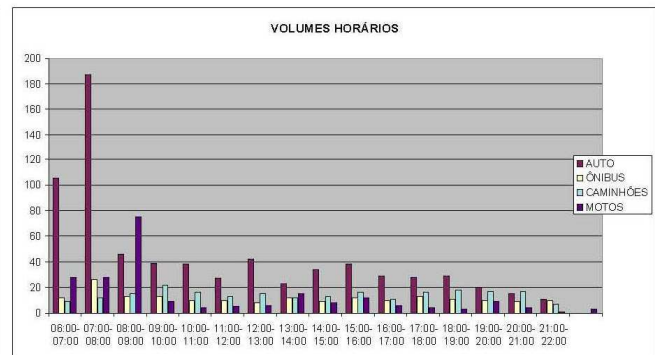
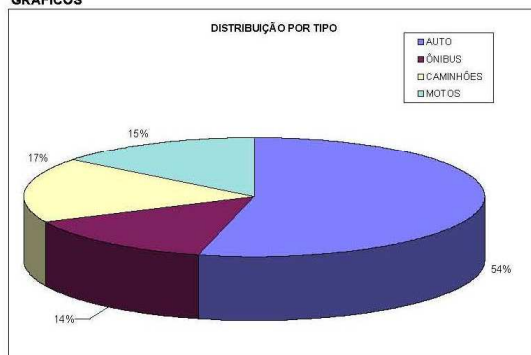
CONTAGEM VOLUMÉTRICA DE VEÍCULOS

VIA: RODOVIA CÔNEGO DOMÊNIGO RANGONI - KM 267,5						SENTIDO: 11 - GUARUJÁ (MARGINAL)						PONTO: 3	
DATA: 27/6/2019						TEMPO		GAROA		TRAFEGO: NORMAL			
HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL		
06:15	8	1	1	1	11	14:15	5	2	3	5	15		
06:30	14	1	1	1	17	14:30	9	0	2	2	13		
06:45	25	5	4	6	40	14:45	9	2	4	1	16		
07:00	58	5	3	20	86	15:00	11	5	4	4	24		
07:15	60	5	5	30	100	15:15	6	2	4	1	13		
07:30	46	5	3	27	81	15:30	8	2	2	0	12		
07:45	40	5	2	9	56	15:45	8	2	3	3	16		
08:00	41	11	2	9	63	16:00	16	6	7	2	31		
08:15	10	4	2	3	29	16:15	8	1	4	1	14		
08:30	11	2	6	3	22	16:30	5	3	2	1	11		
08:45	8	3	4	2	17	16:45	8	5	4	1	18		
09:00	8	4	3	1	16	17:00	8	1	1	1	11		
09:15	10	3	7	0	20	17:15	7	3	5	0	15		
09:30	10	7	3	1	21	17:30	7	6	5	1	19		
09:45	12	2	5	1	20	17:45	9	1	2	1	13		
10:00	7	1	7	2	17	18:00	5	3	4	1	13		
10:15	6	4	6	1	17	18:15	6	3	5	1	15		
10:30	12	1	4	1	18	18:30	5	4	4	4	17		
10:45	7	1	1	1	10	18:45	7	1	4	1	13		
11:00	13	4	5	2	24	19:00	11	3	5	3	22		
11:15	4	2	2	0	8	19:15	7	3	3	0	13		
11:30	9	3	1	3	16	19:30	5	2	1	1	9		
11:45	7	1	2	0	10	19:45	5	3	10	3	21		
12:00	7	4	8	3	22	20:00	3	2	3	0	8		
12:15	12	2	6	3	23	20:15	1	4	5	0	10		
12:30	10	2	4	2	18	20:30	7	1	6	1	15		
12:45	10	2	5	7	24	20:45	4	2	3	0	9		
13:00	10	2	0	3	15	21:00	3	2	3	0	8		
13:15	7	4	2	2	15	21:15	1	3	1	1	6		
13:30	5	2	2	1	10	21:30	4	2	2	1	9		
13:45	5	3	5	4	17	21:45	5	1	3	0	9		
14:00	6	3	3	1	13	22:00	1	4	1	1	7		
SUBTOTAL					875	SUBTOTAL					445		
TOTAL GERAL					1320								

VOLUME HORÁRIO

HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL	HORA	AUTO	ÔNIBUS	CAMINHÕES	MOTOS	TOTAL
06:00-07:00	105	12	9	28	154	14:00-15:00	34	9	13	12	68
07:00-08:00	187	26	12	75	300	15:00-16:00	38	12	16	6	72
08:00-09:00	46	13	15	9	83	16:00-17:00	29	10	11	4	54
09:00-10:00	39	13	22	4	78	17:00-18:00	28	13	16	3	60
10:00-11:00	38	10	16	5	69	18:00-19:00	29	11	18	9	67
11:00-12:00	27	10	13	6	56	19:00-20:00	20	10	17	4	51
12:00-13:00	42	8	15	15	80	20:00-21:00	15	9	17	1	42
13:00-14:00	23	12	12	8	55	21:00-22:00	11	10	7	3	31
SUBTOTAL	507	104	114	150	875	SUBTOTAL	204	84	115	42	445
TOTAL GERAL	711	188	229	192	1320						

GRÁFICOS



ANEXO 02

Relatórios do HCS 2010

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C1		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 1 - Situação Atual			Analysis Year		2019		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			623		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			760		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			164		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	760	0.95	Level	51	0	0.797	1.00	1004	
Ramp	164	0.95	Rolling	13	0	0.837	1.00	206	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 1004 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1210	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1210	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 V_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 10.9 (pc/mi/ln) LOS = B (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.303 (Exhibit 13-11) S _R = 60.1 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 60.1 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. C6nego Dom6nico Rangoni		
Agency or Company					Junction		C1		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cen6rio 2 - Implanta76o			Analysis Year		2020		
Project Description Estudo de Tr6fego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			623		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			791		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			168		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	791	0.95	Level	51	0	0.797	1.00	1045	
Ramp	168	0.95	Level	13	0	0.939	1.00	188	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 1045 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1233	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1233	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = 11.1 (pc/mi/ln) LOS = B (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.303 (Exhibit 13-11) S _R = 60.1 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 60.1 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C1		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 3 - Operação Futura			Analysis Year		2023		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			623		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			833		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			180		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	833	0.95	Level	51	0	0.797	1.00	1100	
Ramp	180	0.95	Level	13	0	0.939	1.00	202	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 1100 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1302	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1302	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = 11.6 (pc/mi/ln) LOS = B (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.304 (Exhibit 13-11) S _R = 60.1 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 60.1 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C1		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 4 - Operação Futura			Analysis Year		2033		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			623		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			1046		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			226		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1046	0.95	Level	51	0	0.797	1.00	1382	
Ramp	226	0.95	Level	13	0	0.939	1.00	253	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 1382 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1635	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1635	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = 14.2 (pc/mi/ln) LOS = B (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.310 (Exhibit 13-11) S _R = 59.9 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 59.9 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C2		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 1 - Situação Atual			Analysis Year		2019		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			426		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			786		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			37		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	786	0.95	Level	49	0	0.803	1.00	1030	
Ramp	37	0.95	Level	73	0	0.733	1.00	53	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 1030 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R) P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1083	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1083	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = 11.2 (pc/mi/ln) LOS = B (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S	0.311 (Exhibit 13-11)				D _S	(Exhibit 13-12)			
S _R	59.9 mph (Exhibit 13-11)				S _R	mph (Exhibit 13-12)			
S ₀	N/A mph (Exhibit 13-11)				S ₀	mph (Exhibit 13-12)			
S	59.9 mph (Exhibit 13-13)				S	mph (Exhibit 13-13)			

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C2		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 2 - Implantação			Analysis Year		2020		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			426		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			804		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			51		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	804	0.95	Level	49	0	0.803	1.00	1054	
Ramp	51	0.95	Level	70	0	0.741	1.00	72	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13)				
L _{EQ} = 1.000 using Equation (Exhibit 13-6)					L _{EQ} = using Equation (Exhibit 13-7)				
P _{FM} = 1054 pc/h					P _{FD} = pc/h				
V ₁₂ = 0 pc/h (Equation 13-14 or 13-17)					V ₁₂ = pc/h (Equation 13-14 or 13-17)				
Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No				
Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No					Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No				
If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1126	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1126	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$				
D _R = 11.6 (pc/mi/ln)					D _R = (pc/mi/ln)				
LOS = B (Exhibit 13-2)					LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.312 (Exhibit 13-11)					D _S = (Exhibit 13-12)				
S _R = 59.9 mph (Exhibit 13-11)					S _R = mph (Exhibit 13-12)				
S ₀ = N/A mph (Exhibit 13-11)					S ₀ = mph (Exhibit 13-12)				
S = 59.9 mph (Exhibit 13-13)					S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C2		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 3 - Operação Futura			Analysis Year		2023		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			426		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			861		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			41		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	861	0.95	Level	49	0	0.803	1.00	1128	
Ramp	41	0.95	Level	73	0	0.733	1.00	59	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 1128 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R) P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1187	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1187	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = 12.0 (pc/mi/ln) LOS = B (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.312 (Exhibit 13-11) S _R = 59.9 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 59.9 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C2		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 4 - Operação Futura			Analysis Year		2033		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2			Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			426			<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D						<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = ft		Freeway Volume, V _F			1081			L _{down} = ft	
V _u = veh/h		Ramp Volume, V _R			51			V _D = veh/h	
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1081	0.95	Level	49	0	0.803	1.00	1417	
Ramp	51	0.95	Level	73	0	0.733	1.00	73	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 1417 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1490	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1490	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 14.4 (pc/mi/ln) LOS = B (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.317 (Exhibit 13-11) S _R = 59.8 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 59.8 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C3		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 1 - Situação Atual			Analysis Year		2019		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			1500		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			680		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			113		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	680	0.95	Level	47	0	0.810	1.00	884	
Ramp	113	0.95	Level	62	0	0.763	1.00	156	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 884 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1040	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1040	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 4.1 (pc/mi/ln) LOS = A (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.257 (Exhibit 13-11) S _R = 61.3 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 61.3 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C3		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 2- Implantação			Analysis Year		2020		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			1500		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			709		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			129		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	709	0.95	Level	47	0	0.810	1.00	922	
Ramp	129	0.95	Level	62	0	0.763	1.00	178	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 922 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1100	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1100	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 4.6 (pc/mi/ln) LOS = A (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.258 (Exhibit 13-11) S _R = 61.3 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 61.3 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C3		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 3 - Operação Futura			Analysis Year		2023		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2			Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			1500			<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D						<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = ft		Freeway Volume, V _F			745			L _{down} = ft	
V _u = veh/h		Ramp Volume, V _R			124			V _D = veh/h	
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	745	0.95	Level	47	0	0.810	1.00	969	
Ramp	124	0.95	Level	62	0	0.763	1.00	171	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 969 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R) P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1140	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1140	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 4.9 (pc/mi/ln) LOS = A (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.258 (Exhibit 13-11) S _R = 61.3 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 61.3 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C3		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 4 - Operação Futura			Analysis Year		2033		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			1500		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			935		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			155		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	935	0.95	Level	47	0	0.810	1.00	1216	
Ramp	155	0.95	Level	62	0	0.763	1.00	214	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 1216 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R) P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1430	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1430	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = 7.1 (pc/mi/ln) LOS = A (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.262 (Exhibit 13-11) S _R = 61.2 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 61.2 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C4		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 1 - Situação Atual			Analysis Year		2019		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			1312		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			682		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			186		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	682	0.95	Level	47	0	0.810	1.00	887	
Ramp	186	0.95	Level	25	0	0.889	1.00	220	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 887 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R) P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1107	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1107	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = 5.8 (pc/mi/ln) LOS = A (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.267 (Exhibit 13-11) S _R = 61.1 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 61.1 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. C6nego Dom6nico Rangoni		
Agency or Company					Junction		C4		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cen6rio 2 - Implanta76o			Analysis Year		2020		
Project Description Estudo de Tr6fego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			1312		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			702		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			190		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	702	0.95	Level	47	0	0.810	1.00	913	
Ramp	190	0.95	Level	25	0	0.889	1.00	225	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 913 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1138	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1138	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 v_{12} - 0.00627 L_A$ D _R = 6.0 (pc/mi/ln) LOS = A (Exhibit 13-2)					$D_R = 4.252 + 0.0086 v_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.268 (Exhibit 13-11) S _R = 61.0 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 61.0 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C4		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 3 - Operação Futura			Analysis Year		2023		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			1312		<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D					<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			747		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			204		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	747	0.95	Level	47	0	0.810	1.00	971	
Ramp	204	0.95	Level	25	0	0.889	1.00	242	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 971 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R) P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1213	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1213	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 6.6 (pc/mi/ln) LOS = A (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.269 (Exhibit 13-11) S _R = 61.0 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 61.0 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		C4		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 4 - Operação Futura			Analysis Year		2033		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2			Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A			1312			<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D						<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = ft		Freeway Volume, V _F			938			L _{down} = ft	
V _u = veh/h		Ramp Volume, V _R			256			V _D = veh/h	
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	938	0.95	Level	47	0	0.810	1.00	1219	
Ramp	256	0.95	Level	25	0	0.889	1.00	303	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = 1.000 using Equation (Exhibit 13-6) V ₁₂ = 1219 pc/h V ₃ or V _{av34} = 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = using Equation (Exhibit 13-7) V ₁₂ = pc/h V ₃ or V _{av34} = pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}	1522	Exhibit 13-8		No	V _F		Exhibit 13-8		
					V _{FO} = V _F - V _R		Exhibit 13-8		
					V _R		Exhibit 13-10		
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}	1522	Exhibit 13-8		No	V ₁₂		Exhibit 13-8		
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = 9.0 (pc/mi/ln) LOS = A (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = 0.273 (Exhibit 13-11) S _R = 60.9 mph (Exhibit 13-11) S ₀ = N/A mph (Exhibit 13-11) S = 60.9 mph (Exhibit 13-13)					D _S = (Exhibit 13-12) S _R = mph (Exhibit 13-12) S ₀ = mph (Exhibit 13-12) S = mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. C6nego Dom6nico Rangoni		
Agency or Company					Junction		D1		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cen6rio 1 - Situa76o Atual			Analysis Year		2019		
Project Description Estudo de Tr6fego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2			Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A						<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			490			<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = ft		Freeway Volume, V _F			823			L _{down} = ft	
V _u = veh/h		Ramp Volume, V _R			277			V _D = veh/h	
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	823	0.95	Level	50	0	0.800	1.00	1083	
Ramp	277	0.95	Level	65	0	0.755	1.00	386	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1083 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1083	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	697	Exhibit 13-8		4760 No
					V _R	386	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1083	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 9.2 (pc/mi/ln) LOS = A (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _S = 0.593 (Exhibit 13-12) S _R = 52.6 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 52.6 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. C6nego Dom6nico Rangoni		
Agency or Company					Junction		D1		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cen6rio 2 - Implanta76o			Analysis Year		2020		
Project Description Estudo de Tr6fego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A					<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			490		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			855		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			287		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	855	0.95	Level	50	0	0.800	1.00	1125	
Ramp	287	0.95	Level	65	0	0.755	1.00	400	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1125 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1125	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	725	Exhibit 13-8		4760 No
					V _R	400	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1125	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 9.5 (pc/mi/ln) LOS = A (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _S = 0.594 (Exhibit 13-12) S _R = 52.6 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 52.6 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. C6nego Dom6nico Rangoni		
Agency or Company					Junction		D1		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cen6rio 3 - Opera76o Futura			Analysis Year		2023		
Project Description Estudo de Tr6fego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A					<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			490		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			902		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			303		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	902	0.95	Level	50	0	0.800	1.00	1187	
Ramp	303	0.95	Level	65	0	0.755	1.00	423	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1187 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1187	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	764	Exhibit 13-8		4760 No
					V _R	423	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1187	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 10.1 (pc/mi/ln) LOS = B (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _S = 0.596 (Exhibit 13-12) S _R = 52.5 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 52.5 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. C6nego Dom6nico Rangoni		
Agency or Company					Junction		D1		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cen6rio 4 - Opera76o Futura			Analysis Year		2033		
Project Description Estudo de Tr6fego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2			Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A						<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			490			<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = ft		Freeway Volume, V _F			1132			L _{down} = ft	
V _u = veh/h		Ramp Volume, V _R			381			V _D = veh/h	
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1132	0.95	Level	50	0	0.800	1.00	1489	
Ramp	381	0.95	Level	65	0	0.755	1.00	531	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1489 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1489	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	958	Exhibit 13-8		4760 No
					V _R	531	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1489	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 12.6 (pc/mi/ln) LOS = B (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _S = 0.606 (Exhibit 13-12) S _R = 52.2 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 52.2 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. C6nego Dom6nico Rangoni		
Agency or Company					Junction		D3		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cen6rio 1 - Situa76o Atual			Analysis Year		2019		
Project Description Estudo de Tr6fego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2			Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A						<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			525			<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = ft		Freeway Volume, V _F			815			L _{down} = ft	
V _u = veh/h		Ramp Volume, V _R			29			V _D = veh/h	
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	815	0.95	Level	50	0	0.800	1.00	1072	
Ramp	29	0.95	Level	76	0	0.725	1.00	42	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1072 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1072	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	1030	Exhibit 13-8		4760 No
					V _R	42	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1072	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 8.7 (pc/mi/ln) LOS = A (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _S = 0.562 (Exhibit 13-12) S _R = 53.4 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 53.4 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET										
General Information					Site Information					
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni			
Agency or Company					Junction		D3			
Date Performed		02/10/2019			Jurisdiction					
Analysis Time Period		Cenário 2 - Implantação			Analysis Year		2020			
Project Description Estudo de Tráfego - URE Valoriza										
Inputs										
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp			
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A					<input type="checkbox"/> Yes <input type="checkbox"/> On			
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			525		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off			
L _{up} = ft		Freeway Volume, V _F			847		L _{down} = ft			
V _u = veh/h		Ramp Volume, V _R			43		V _D = veh/h			
		Freeway Free-Flow Speed, S _{FF}			68.0					
		Ramp Free-Flow Speed, S _{FR}			25.0					
Conversion to pc/h Under Base Conditions										
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p		
Freeway	847	0.95	Level	50	0	0.800	1.00	1114		
Ramp	43	0.95	Level	71	0	0.738	1.00	61		
UpStream										
DownStream										
Merge Areas					Diverge Areas					
Estimation of v ₁₂					Estimation of v ₁₂					
L _{EQ} =		V ₁₂ = V _F (P _{FM})			L _{EQ} =		V ₁₂ = V _R + (V _F - V _R)P _{FD}			
P _{FM} =		(Equation 13-6 or 13-7)			P _{FD} =		(Equation 13-12 or 13-13)			
V ₁₂ =		using Equation (Exhibit 13-6)			V ₁₂ =		1.000 using Equation (Exhibit 13-7)			
V ₃ or V _{av34}		pc/h (Equation 13-14 or 13-17)			V ₃ or V _{av34}		1114 pc/h			
Is V ₃ or V _{av34} > 2,700 pc/h?		<input type="checkbox"/> Yes <input type="checkbox"/> No			Is V ₃ or V _{av34} > 2,700 pc/h?		<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No			
Is V ₃ or V _{av34} > 1.5 * V _{12/2} ?		<input type="checkbox"/> Yes <input type="checkbox"/> No			Is V ₃ or V _{av34} > 1.5 * V _{12/2} ?		<input type="checkbox"/> Yes <input checked="" type="checkbox"/> No			
If Yes, V _{12a} =		pc/h (Equation 13-16, 13-18, or 13-19)			If Yes, V _{12a} =		pc/h (Equation 13-16, 13-18, or 13-19)			
Capacity Checks					Capacity Checks					
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?	
V _{FO}		Exhibit 13-8			V _F	1114	Exhibit 13-8		4760	No
					V _{FO} = V _F - V _R	1053	Exhibit 13-8		4760	No
					V _R	61	Exhibit 13-10		1900	No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area					
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?	
V _{R12}		Exhibit 13-8			V ₁₂	1114	Exhibit 13-8		4400:All	No
Level of Service Determination (if not F)					Level of Service Determination (if not F)					
D _R = 5.475 + 0.00734 v _R + 0.0078 V ₁₂ - 0.00627 L _A					D _R = 4.252 + 0.0086 V ₁₂ - 0.009 L _D					
D _R = (pc/mi/lh)					D _R = 9.1 (pc/mi/lh)					
LOS = (Exhibit 13-2)					LOS = A (Exhibit 13-2)					
Speed Determination					Speed Determination					
M _S = (Exhibit 13-11)					D _S = 0.563 (Exhibit 13-12)					
S _R = mph (Exhibit 13-11)					S _R = 53.3 mph (Exhibit 13-12)					
S ₀ = mph (Exhibit 13-11)					S ₀ = N/A mph (Exhibit 13-12)					
S = mph (Exhibit 13-13)					S = 53.3 mph (Exhibit 13-13)					

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. C6nego Dom6nico Rangoni		
Agency or Company					Junction		D3		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cen6rio 3 - Opera76o Futura			Analysis Year		2023		
Project Description Estudo de Tr6fego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2			Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A						<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			525			<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = ft		Freeway Volume, V _F			893			L _{down} = ft	
V _u = veh/h		Ramp Volume, V _R			32			V _D = veh/h	
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	893	0.95	Level	50	0	0.800	1.00	1175	
Ramp	32	0.95	Level	76	0	0.725	1.00	46	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1175 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1175	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	1129	Exhibit 13-8		4760 No
					V _R	46	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1175	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 9.6 (pc/mi/ln) LOS = A (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _S = 0.562 (Exhibit 13-12) S _R = 53.4 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 53.4 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		D3		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 4 - Operação Futura			Analysis Year		2033		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A					<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			525		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			1121		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			40		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1121	0.95	Level	50	0	0.800	1.00	1475	
Ramp	40	0.95	Level	76	0	0.725	1.00	58	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1475 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1475	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	1417	Exhibit 13-8		4760 No
					V _R	58	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1475	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 12.2 (pc/mi/ln) LOS = B (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _S = 0.563 (Exhibit 13-12) S _R = 53.4 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 53.4 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		D4		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 1 - Situação Atual			Analysis Year		2019		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A					<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			525		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			924		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			242		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	924	0.95	Level	44	0	0.820	1.00	1187	
Ramp	242	0.95	Level	36	0	0.847	1.00	301	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1187 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1187	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	886	Exhibit 13-8		4760 No
					V _R	301	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1187	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/lh) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 9.7 (pc/mi/lh) LOS = A (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _S = 0.585 (Exhibit 13-12) S _R = 52.8 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 52.8 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. Cônego Domênico Rangoni		
Agency or Company					Junction		D4		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cenário 2 - Implantação			Analysis Year		2020		
Project Description Estudo de Tráfego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A					<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			525		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			958		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			257		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	958	0.95	Level	44	0	0.820	1.00	1230	
Ramp	257	0.95	Level	37	0	0.844	1.00	321	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1230 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1230	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	909	Exhibit 13-8		4760 No
					V _R	321	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1230	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 10.1 (pc/mi/ln) LOS = B (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _S = 0.587 (Exhibit 13-12) S _R = 52.7 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 52.7 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. C6nego Dom6nico Rangoni		
Agency or Company					Junction		D4		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cen6rio 3 - Opera76o Futura			Analysis Year		2023		
Project Description Estudo de Tr6fego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2		Downstream Adj Ramp		
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A					<input type="checkbox"/> Yes <input type="checkbox"/> On		
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			525		<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		
L _{up} = ft		Freeway Volume, V _F			1012		L _{down} = ft		
V _u = veh/h		Ramp Volume, V _R			265		V _D = veh/h		
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1012	0.95	Level	44	0	0.820	1.00	1300	
Ramp	265	0.95	Level	36	0	0.847	1.00	329	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) L _{EQ} = P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) L _{EQ} = P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1300 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1300	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	971	Exhibit 13-8		4760 No
					V _R	329	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1300	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 10.7 (pc/mi/ln) LOS = B (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _S = (Exhibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _S = 0.588 (Exhibit 13-12) S _R = 52.7 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 52.7 mph (Exhibit 13-13)				

RAMPS AND RAMP JUNCTIONS WORKSHEET									
General Information					Site Information				
Analyst		TTC Engenharia			Freeway/Dir of Travel		Rod. C6nego Dom6nico Rangoni		
Agency or Company					Junction		D4		
Date Performed		02/10/2019			Jurisdiction				
Analysis Time Period		Cen6rio 4 - Opera76o Futura			Analysis Year		2033		
Project Description Estudo de Tr6fego - URE Valoriza									
Inputs									
Upstream Adj Ramp		Number of Lanes, N			2			Downstream Adj Ramp	
<input type="checkbox"/> Yes <input type="checkbox"/> On		Acceleration Lane Length, L _A						<input type="checkbox"/> Yes <input type="checkbox"/> On	
<input checked="" type="checkbox"/> No <input type="checkbox"/> Off		Deceleration Lane Length L _D			525			<input checked="" type="checkbox"/> No <input type="checkbox"/> Off	
L _{up} = ft		Freeway Volume, V _F			1271			L _{down} = ft	
V _u = veh/h		Ramp Volume, V _R			333			V _D = veh/h	
		Freeway Free-Flow Speed, S _{FF}			68.0				
		Ramp Free-Flow Speed, S _{FR}			25.0				
Conversion to pc/h Under Base Conditions									
(pc/h)	V (Veh/hr)	PHF	Terrain	%Truck	%Rv	f _{HV}	f _p	v = V/PHF x f _{HV} x f _p	
Freeway	1271	0.95	Level	44	0	0.820	1.00	1632	
Ramp	333	0.95	Level	36	0	0.847	1.00	414	
UpStream									
DownStream									
Merge Areas					Diverge Areas				
Estimation of v ₁₂					Estimation of v ₁₂				
$V_{12} = V_F (P_{FM})$ (Equation 13-6 or 13-7) P _{FM} = using Equation (Exhibit 13-6) V ₁₂ = pc/h V ₃ or V _{av34} pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)					$V_{12} = V_R + (V_F - V_R)P_{FD}$ (Equation 13-12 or 13-13) P _{FD} = 1.000 using Equation (Exhibit 13-7) V ₁₂ = 1632 pc/h V ₃ or V _{av34} 0 pc/h (Equation 13-14 or 13-17) Is V ₃ or V _{av34} > 2,700 pc/h? <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No Is V ₃ or V _{av34} > 1.5 * V ₁₂ /2 <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No If Yes, V _{12a} = pc/h (Equation 13-16, 13-18, or 13-19)				
Capacity Checks					Capacity Checks				
	Actual	Capacity		LOS F?		Actual	Capacity		LOS F?
V _{FO}		Exhibit 13-8			V _F	1632	Exhibit 13-8		4760 No
					V _{FO} = V _F - V _R	1218	Exhibit 13-8		4760 No
					V _R	414	Exhibit 13-10		1900 No
Flow Entering Merge Influence Area					Flow Entering Diverge Influence Area				
	Actual	Max Desirable		Violation?		Actual	Max Desirable		Violation?
V _{R12}		Exhibit 13-8			V ₁₂	1632	Exhibit 13-8		4400:All No
Level of Service Determination (if not F)					Level of Service Determination (if not F)				
$D_R = 5.475 + 0.00734 v_R + 0.0078 V_{12} - 0.00627 L_A$ D _R = (pc/mi/ln) LOS = (Exhibit 13-2)					$D_R = 4.252 + 0.0086 V_{12} - 0.009 L_D$ D _R = 13.6 (pc/mi/ln) LOS = B (Exhibit 13-2)				
Speed Determination					Speed Determination				
M _s = (Exhibit 13-11) S _R = mph (Exhibit 13-11) S ₀ = mph (Exhibit 13-11) S = mph (Exhibit 13-13)					D _s = 0.595 (Exhibit 13-12) S _R = 52.5 mph (Exhibit 13-12) S ₀ = N/A mph (Exhibit 13-12) S = 52.5 mph (Exhibit 13-13)				

Estudo de Tráfego - URE Valoriza

FREEWAY WEAVING WORKSHEET									
General Information					Site Information				
Analyst	TTC Engenharia				Freeway/Dir of Travel	Rod. Cônego Domênico Rangoni			
Agency/Company					Weaving Segment Location	E1			
Date Performed	02/10/2019				Analysis Year	2019			
Analysis Time Period	Cenário 1 - Situação Atual								
Project Description <i>Estudo de Tráfego - URE Valoriza</i>									
Inputs									
Weaving configuration	One-Sided				Segment type	C-D Roadway/			
Weaving number of lanes, N	3					Multilane			
Weaving segment length, L_S	1772ft				Freeway minimum speed, S_{MIN}	25			
Freeway free-flow speed, FFS	68 mph				Freeway maximum capacity, C_{IFL}	2400			
					Terrain type	Level			
Conversions to pc/h Under Base Conditions									
	V (veh/h)	PHF	Truck (%)	RV (%)	E_T	E_R	f_{HV}	f_p	v (pc/h)
V_{FF}	564	0.95	38	0	1.5	1.2	0.840	1.00	706
V_{RF}	194	0.95	89	0	1.5	1.2	0.692	1.00	295
V_{FR}	4	0.95	0	0	1.5	1.2	1.000	1.00	4
V_{RR}	0	0.95	0	0	1.5	1.2	1.000	1.00	0
V_{NW}	706							V =	1005
V_W	299								
VR	0.298								
Configuration Characteristics									
Minimum maneuver lanes, N_{WL}	2 lc				Minimum weaving lane changes, LC_{MIN}	299 lc/h			
Interchange density, ID	0.0 int/mi				Weaving lane changes, LC_W	434 lc/h			
Minimum RF lane changes, LC_{RF}	1 lc/pc				Non-weaving lane changes, LC_{NW}	528 lc/h			
Minimum FR lane changes, LC_{FR}	1 lc/pc				Total lane changes, LC_{ALL}	962 lc/h			
Minimum RR lane changes, LC_{RR}	lc/pc				Non-weaving vehicle index, I_{NW}	0			
Weaving Segment Speed, Density, Level of Service, and Capacity									
Weaving segment flow rate, v	1005 pc/h				Weaving intensity factor, W	0.140			
Weaving segment capacity, c_w	5319 veh/h				Weaving segment speed, S	63.8 mph			
Weaving segment v/c ratio	0.159				Average weaving speed, S_W	62.7 mph			
Weaving segment density, D	5.3 pc/mi/ln				Average non-weaving speed, S_{NW}	64.2 mph			
Level of Service, LOS	A				Maximum weaving length, L_{MAX}	5557 ft			
Notes									
a. Weaving segments longer than the calculated maximum length should be treated as isolated merge and diverge areas using the procedures of Chapter 13, "Freeway Merge and Diverge Segments".									
b. For volumes that exceed the weaving segment capacity, the level of service is "F".									

Estudo de Tráfego - URE Valoriza

FREEWAY WEAVING WORKSHEET									
General Information					Site Information				
Analyst	TTC Engenharia				Freeway/Dir of Travel	Rod. Cônego Domênico Rangoni			
Agency/Company					Weaving Segment Location	E1			
Date Performed	02/10/2019				Analysis Year	2020			
Analysis Time Period	Cenário 2 - Implantação								
Project Description <i>Estudo de Tráfego - URE Valoriza</i>									
Inputs									
Weaving configuration	One-Sided				Segment type	C-D Roadway/			
Weaving number of lanes, N	3					Multilane			
Weaving segment length, L_S	1772ft				Freeway minimum speed, S_{MIN}	25			
Freeway free-flow speed, FFS	68 mph				Freeway maximum capacity, C_{IFL}	2400			
					Terrain type	Level			
Conversions to pc/h Under Base Conditions									
	V (veh/h)	PHF	Truck (%)	RV (%)	E_T	E_R	f_{HV}	f_p	v (pc/h)
V_{FF}	577	0.95	38	0	1.5	1.2	0.840	1.00	723
V_{RF}	211	0.95	87	0	1.5	1.2	0.697	1.00	319
V_{FR}	4	0.95	0	0	1.5	1.2	1.000	1.00	4
V_{RR}	0	0.95	0	0	1.5	1.2	1.000	1.00	0
V_{NW}	723							V =	1046
V_W	323								
VR	0.309								
Configuration Characteristics									
Minimum maneuver lanes, N_{WL}	2 lc				Minimum weaving lane changes, LC_{MIN}	323 lc/h			
Interchange density, ID	0.0 int/mi				Weaving lane changes, LC_W	458 lc/h			
Minimum RF lane changes, LC_{RF}	1 lc/pc				Non-weaving lane changes, LC_{NW}	532 lc/h			
Minimum FR lane changes, LC_{FR}	1 lc/pc				Total lane changes, LC_{ALL}	990 lc/h			
Minimum RR lane changes, LC_{RR}	lc/pc				Non-weaving vehicle index, I_{NW}	0			
Weaving Segment Speed, Density, Level of Service, and Capacity									
Weaving segment flow rate, v	1046 pc/h				Weaving intensity factor, W	0.143			
Weaving segment capacity, c_w	5297 veh/h				Weaving segment speed, S	63.6 mph			
Weaving segment v/c ratio	0.166				Average weaving speed, S_W	62.6 mph			
Weaving segment density, D	5.5 pc/mi/ln				Average non-weaving speed, S_{NW}	64.0 mph			
Level of Service, LOS	A				Maximum weaving length, L_{MAX}	5678 ft			
Notes									
a. Weaving segments longer than the calculated maximum length should be treated as isolated merge and diverge areas using the procedures of Chapter 13, "Freeway Merge and Diverge Segments".									
b. For volumes that exceed the weaving segment capacity, the level of service is "F".									

Estudo de Tráfego - URE Valoriza

FREEWAY WEAVING WORKSHEET									
General Information					Site Information				
Analyst	TTC Engenharia				Freeway/Dir of Travel	Rod. Cônego Domênico Rangoni			
Agency/Company					Weaving Segment Location	E1			
Date Performed	02/10/2019				Analysis Year	2023			
Analysis Time Period	Cenário 3 - Operação Futura								
Project Description <i>Estudo de Tráfego - URE Valoriza</i>									
Inputs									
Weaving configuration	One-Sided				Segment type	C-D Roadway/			
Weaving number of lanes, N	3					Multilane			
Weaving segment length, L_S	1772ft				Freeway minimum speed, S_{MIN}	25			
Freeway free-flow speed, FFS	68 mph				Freeway maximum capacity, C_{IFL}	2400			
					Terrain type	Level			
Conversions to pc/h Under Base Conditions									
	V (veh/h)	PHF	Truck (%)	RV (%)	E_T	E_R	f_{HV}	f_p	v (pc/h)
V_{FF}	618	0.95	38	0	1.5	1.2	0.840	1.00	774
V_{RF}	213	0.95	89	0	1.5	1.2	0.692	1.00	324
V_{FR}	4	0.95	0	0	1.5	1.2	1.000	1.00	4
V_{RR}	0	0.95	0	0	1.5	1.2	1.000	1.00	0
V_{NW}	774							V =	1102
V_W	328								
VR	0.298								
Configuration Characteristics									
Minimum maneuver lanes, N_{WL}	2 lc				Minimum weaving lane changes, LC_{MIN}	328 lc/h			
Interchange density, ID	0.0 int/mi				Weaving lane changes, LC_W	463 lc/h			
Minimum RF lane changes, LC_{RF}	1 lc/pc				Non-weaving lane changes, LC_{NW}	542 lc/h			
Minimum FR lane changes, LC_{FR}	1 lc/pc				Total lane changes, LC_{ALL}	1005 lc/h			
Minimum RR lane changes, LC_{RR}	lc/pc				Non-weaving vehicle index, I_{NW}	0			
Weaving Segment Speed, Density, Level of Service, and Capacity									
Weaving segment flow rate, v	1102 pc/h				Weaving intensity factor, W	0.144			
Weaving segment capacity, c_w	5319 veh/h				Weaving segment speed, S	63.5 mph			
Weaving segment v/c ratio	0.174				Average weaving speed, S_W	62.6 mph			
Weaving segment density, D	5.8 pc/mi/ln				Average non-weaving speed, S_{NW}	63.9 mph			
Level of Service, LOS	A				Maximum weaving length, L_{MAX}	5559 ft			
Notes									
a. Weaving segments longer than the calculated maximum length should be treated as isolated merge and diverge areas using the procedures of Chapter 13, "Freeway Merge and Diverge Segments".									
b. For volumes that exceed the weaving segment capacity, the level of service is "F".									

Estudo de Tráfego - URE Valoriza

FREEWAY WEAVING WORKSHEET									
General Information					Site Information				
Analyst	TTC Engenharia				Freeway/Dir of Travel	Rod. Cônego Domênico Rangoni			
Agency/Company					Weaving Segment Location	E1			
Date Performed	02/10/2019				Analysis Year	2033			
Analysis Time Period	Cenário 4 - Operação Futura								
Project Description <i>Estudo de Tráfego - URE Valoriza</i>									
Inputs									
Weaving configuration	One-Sided				Segment type	C-D Roadway/			
Weaving number of lanes, N	3					Multilane			
Weaving segment length, L_S	1772ft				Freeway minimum speed, S_{MIN}	25			
Freeway free-flow speed, FFS	68 mph				Freeway maximum capacity, C_{IFL}	2400			
					Terrain type	Level			
Conversions to pc/h Under Base Conditions									
	V (veh/h)	PHF	Truck (%)	RV (%)	E_T	E_R	f_{HV}	f_p	v (pc/h)
V_{FF}	776	0.95	38	0	1.5	1.2	0.840	1.00	972
V_{RF}	267	0.95	89	0	1.5	1.2	0.692	1.00	406
V_{FR}	6	0.95	0	0	1.5	1.2	1.000	1.00	6
V_{RR}	0	0.95	0	0	1.5	1.2	1.000	1.00	0
V_{NW}	972							V =	1384
V_W	412								
VR	0.298								
Configuration Characteristics									
Minimum maneuver lanes, N_{WL}	2 lc				Minimum weaving lane changes, LC_{MIN}	412 lc/h			
Interchange density, ID	0.0 int/mi				Weaving lane changes, LC_W	547 lc/h			
Minimum RF lane changes, LC_{RF}	1 lc/pc				Non-weaving lane changes, LC_{NW}	583 lc/h			
Minimum FR lane changes, LC_{FR}	1 lc/pc				Total lane changes, LC_{ALL}	1130 lc/h			
Minimum RR lane changes, LC_{RR}	lc/pc				Non-weaving vehicle index, I_{NW}	0			
Weaving Segment Speed, Density, Level of Service, and Capacity									
Weaving segment flow rate, v	1384 pc/h				Weaving intensity factor, W	0.158			
Weaving segment capacity, c_w	5319 veh/h				Weaving segment speed, S	62.6 mph			
Weaving segment v/c ratio	0.219				Average weaving speed, S_W	62.1 mph			
Weaving segment density, D	7.4 pc/mi/ln				Average non-weaving speed, S_{NW}	62.8 mph			
Level of Service, LOS	A				Maximum weaving length, L_{MAX}	5559 ft			
Notes									
a. Weaving segments longer than the calculated maximum length should be treated as isolated merge and diverge areas using the procedures of Chapter 13, "Freeway Merge and Diverge Segments".									
b. For volumes that exceed the weaving segment capacity, the level of service is "F".									

MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N, v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																					
Agency or Company		From/To	SP056 - Pedágio																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 1 - Situação Atual	Analysis Year	2019																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																						
<input type="checkbox"/> Plan. (v_p)																								
Flow Inputs																								
Volume, V (veh/h)	1128	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Level																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	1.2																					
E_T	1.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	50.0																					
Base Free-Flow Speed, BFFS	50.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/lane)	593	Required Number of Lanes, N																						
Speed, S (mi/h)	50.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/h)	11.9	Max Service Flow Rate (pc/h/lane)																						
LOS	B	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			593.7																					
Effective width, W_e (Eq. 15-29) ft			29.00																					
Effective speed factor, S_e (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.16																					
Bicycle level of service (Exhibit 15-4)			B																					

MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N, v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																					
Agency or Company		From/To	SP056 - Pedágio																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 2 - Implantação	Analysis Year	2020																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																						
<input type="checkbox"/> Plan. (vp)																								
Flow Inputs																								
Volume, V (veh/h)	1171	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Level																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	1.2																					
E_T	1.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	50.0																					
Base Free-Flow Speed, BFFS	50.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/ln)	616	Required Number of Lanes, N																						
Speed, S (mi/h)	50.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/ln)	12.3	Max Service Flow Rate (pc/h/ln)																						
LOS	B	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			616.3																					
Effective width, W_e (Eq. 15-29) ft			29.00																					
Effective speed factor, S_e (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.18																					
Bicycle level of service (Exhibit 15-4)			B																					
Copyright © 2012 University of Florida, All Rights Reserved		HCS 2010™ Version 6.3																						
		Generated: 02/10/2019 16:40																						

MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N, v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																					
Agency or Company		From/To	SP056 - Pedágio																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 3 - Operação Futura	Analysis Year	2023																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																						
<input type="checkbox"/> Plan. (v_p)																								
Flow Inputs																								
Volume, V (veh/h)	1236	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Level																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	1.2																					
E_T	1.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	50.0																					
Base Free-Flow Speed, BFFS	50.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/ln)	650	Required Number of Lanes, N																						
Speed, S (mi/h)	50.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/ln)	13.0	Max Service Flow Rate (pc/h/ln)																						
LOS	B	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			650.5																					
Effective width, W_e (Eq. 15-29) ft			29.00																					
Effective speed factor, S_e (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.20																					
Bicycle level of service (Exhibit 15-4)			B																					
Copyright © 2012 University of Florida, All Rights Reserved		HCS 2010™ Version 6.3																						
		Generated: 02/10/2019 16:41																						

MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N, v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information Analyst: TTC Engenharia Agency or Company: Date Performed: 02/10/2019 Analysis Time Period: Cenário 4 - Operação Futura		Site Information Highway/Direction to Travel: Rod. Cônego Domênico Rangoni From/To: SP056 - Pedágio Jurisdiction: Analysis Year: 2033																						
Project Description: Estudo de Tráfego - URE Valoriza <input type="checkbox"/> Oper.(LOS) <input type="checkbox"/> Des. (N) <input type="checkbox"/> Plan. (vp)																								
Flow Inputs																								
Volume, V (veh/h)	1552	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Level																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	1.2																					
E_T	1.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	50.0																					
Base Free-Flow Speed, BFFS	50.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/ln)	816	Required Number of Lanes, N																						
Speed, S (mi/h)	50.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/h)	16.3	Max Service Flow Rate (pc/h/ln)																						
LOS	B	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			816.8																					
Effective width, W_v (Eq. 15-29) ft			29.00																					
Effective speed factor, S_f (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.32																					
Bicycle level of service (Exhibit 15-4)			B																					

MULTILANE HIGHWAYS WORKSHEET(Direction 1)																																	
	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>	Application	Input	Output	Operational (LOS)	FFS, N , v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D											
Application	Input	Output																															
Operational (LOS)	FFS, N , v_p	LOS, S, D																															
Design (N)	FFS, LOS, v_p	N, S, D																															
Design (v_p)	FFS, LOS, N	v_p , S, D																															
Planning (LOS)	FFS, N, AADT	LOS, S, D																															
Planning (N)	FFS, LOS, AADT	N, S, D																															
Planning (v_p)	FFS, LOS, N	v_p , S, D																															
<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">General Information</th> <th colspan="2">Site Information</th> </tr> </thead> <tbody> <tr> <td>Analyst</td> <td>TTC Engenharia</td> <td>Highway/Direction to Travel</td> <td>Rod. Cônego Domênico Rangoni</td> </tr> <tr> <td>Agency or Company</td> <td></td> <td>From/To</td> <td>SP056 - Pedágio</td> </tr> <tr> <td>Date Performed</td> <td>02/10/2019</td> <td>Jurisdiction</td> <td></td> </tr> <tr> <td>Analysis Time Period</td> <td>Cenário 1 - Situação Atual</td> <td>Analysis Year</td> <td>2019</td> </tr> <tr> <td colspan="4">Project Description: Estudo de Tráfego - URE Valoriza</td> </tr> <tr> <td colspan="2" style="text-align: center;"><input type="checkbox"/> Oper.(LOS)</td> <td colspan="2" style="text-align: center;"><input type="checkbox"/> Des. (N)</td> </tr> <tr> <td colspan="4" style="text-align: center;"><input type="checkbox"/> Plan. (v_p)</td> </tr> </tbody> </table>		General Information		Site Information		Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni	Agency or Company		From/To	SP056 - Pedágio	Date Performed	02/10/2019	Jurisdiction		Analysis Time Period	Cenário 1 - Situação Atual	Analysis Year	2019	Project Description: Estudo de Tráfego - URE Valoriza				<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)		<input type="checkbox"/> Plan. (v_p)			
General Information		Site Information																															
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																														
Agency or Company		From/To	SP056 - Pedágio																														
Date Performed	02/10/2019	Jurisdiction																															
Analysis Time Period	Cenário 1 - Situação Atual	Analysis Year	2019																														
Project Description: Estudo de Tráfego - URE Valoriza																																	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																															
<input type="checkbox"/> Plan. (v_p)																																	
Flow Inputs <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td>Volume, V (veh/h)</td> <td>1030</td> <td>Peak-Hour Factor, PHF</td> <td>0.95</td> </tr> <tr> <td>AADT (veh/h)</td> <td></td> <td>%Trucks and Buses, P_T</td> <td>0</td> </tr> <tr> <td>Peak-Hour Prop of AADT (veh/d)</td> <td></td> <td>%RVs, P_R</td> <td>0</td> </tr> <tr> <td>Peak-Hour Direction Prop, D</td> <td></td> <td>General Terrain:</td> <td>Level</td> </tr> <tr> <td>DDHV (veh/h)</td> <td></td> <td>Grade Length (mi)</td> <td>0.00</td> </tr> <tr> <td>Driver Type Adjustment</td> <td>1.00</td> <td>Up/Down %</td> <td>0.00</td> </tr> <tr> <td></td> <td></td> <td>Number of Lanes</td> <td>2</td> </tr> </tbody> </table>				Volume, V (veh/h)	1030	Peak-Hour Factor, PHF	0.95	AADT (veh/h)		%Trucks and Buses, P_T	0	Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0	Peak-Hour Direction Prop, D		General Terrain:	Level	DDHV (veh/h)		Grade Length (mi)	0.00	Driver Type Adjustment	1.00	Up/Down %	0.00			Number of Lanes	2		
Volume, V (veh/h)	1030	Peak-Hour Factor, PHF	0.95																														
AADT (veh/h)		%Trucks and Buses, P_T	0																														
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																														
Peak-Hour Direction Prop, D		General Terrain:	Level																														
DDHV (veh/h)		Grade Length (mi)	0.00																														
Driver Type Adjustment	1.00	Up/Down %	0.00																														
		Number of Lanes	2																														
Calculate Flow Adjustments <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td>f_p</td> <td>1.00</td> <td>E_R</td> <td>1.2</td> </tr> <tr> <td>E_T</td> <td>1.5</td> <td>f_{HV}</td> <td>1.000</td> </tr> </tbody> </table>				f_p	1.00	E_R	1.2	E_T	1.5	f_{HV}	1.000																						
f_p	1.00	E_R	1.2																														
E_T	1.5	f_{HV}	1.000																														
Speed Inputs <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td>Lane Width, LW (ft)</td> <td>12.0</td> </tr> <tr> <td>Total Lateral Clearance, LC (ft)</td> <td>12.0</td> </tr> <tr> <td>Access Points, A (A/mi)</td> <td>0</td> </tr> <tr> <td>Median Type, M</td> <td>Divded</td> </tr> <tr> <td>FFS (measured)</td> <td></td> </tr> <tr> <td>Base Free-Flow Speed, BFFS</td> <td>50.0</td> </tr> </tbody> </table>		Lane Width, LW (ft)	12.0	Total Lateral Clearance, LC (ft)	12.0	Access Points, A (A/mi)	0	Median Type, M	Divded	FFS (measured)		Base Free-Flow Speed, BFFS	50.0	Calc Speed Adj and FFS <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td>f_{W} (mi/h)</td> <td>0.0</td> </tr> <tr> <td>f_{LC} (mi/h)</td> <td>0.0</td> </tr> <tr> <td>f_A (mi/h)</td> <td>0.0</td> </tr> <tr> <td>f_M (mi/h)</td> <td>0.0</td> </tr> <tr> <td>FFS (mi/h)</td> <td>50.0</td> </tr> </tbody> </table>		f_{W} (mi/h)	0.0	f_{LC} (mi/h)	0.0	f_A (mi/h)	0.0	f_M (mi/h)	0.0	FFS (mi/h)	50.0								
Lane Width, LW (ft)	12.0																																
Total Lateral Clearance, LC (ft)	12.0																																
Access Points, A (A/mi)	0																																
Median Type, M	Divded																																
FFS (measured)																																	
Base Free-Flow Speed, BFFS	50.0																																
f_{W} (mi/h)	0.0																																
f_{LC} (mi/h)	0.0																																
f_A (mi/h)	0.0																																
f_M (mi/h)	0.0																																
FFS (mi/h)	50.0																																
Operations <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td colspan="2"><u>Operational (LOS)</u></td> </tr> <tr> <td>Flow Rate, v_p (pc/h/ln)</td> <td>542</td> </tr> <tr> <td>Speed, S (mi/h)</td> <td>50.0</td> </tr> <tr> <td>D (pc/mi/ln)</td> <td>10.8</td> </tr> <tr> <td>LOS</td> <td>A</td> </tr> </tbody> </table>		<u>Operational (LOS)</u>		Flow Rate, v_p (pc/h/ln)	542	Speed, S (mi/h)	50.0	D (pc/mi/ln)	10.8	LOS	A	Design <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td colspan="2"><u>Design (N)</u></td> </tr> <tr> <td>Required Number of Lanes, N</td> <td></td> </tr> <tr> <td>Flow Rate, v_p (pc/h)</td> <td></td> </tr> <tr> <td>Max Service Flow Rate (pc/h/ln)</td> <td></td> </tr> <tr> <td>Design LOS</td> <td></td> </tr> </tbody> </table>		<u>Design (N)</u>		Required Number of Lanes, N		Flow Rate, v_p (pc/h)		Max Service Flow Rate (pc/h/ln)		Design LOS											
<u>Operational (LOS)</u>																																	
Flow Rate, v_p (pc/h/ln)	542																																
Speed, S (mi/h)	50.0																																
D (pc/mi/ln)	10.8																																
LOS	A																																
<u>Design (N)</u>																																	
Required Number of Lanes, N																																	
Flow Rate, v_p (pc/h)																																	
Max Service Flow Rate (pc/h/ln)																																	
Design LOS																																	
Bicycle Level of Service <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td>Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h</td> <td>542.1</td> </tr> <tr> <td>Effective width, W_e (Eq. 15-29) ft</td> <td>29.00</td> </tr> <tr> <td>Effective speed factor, S_e (Eq. 15-30)</td> <td>4.79</td> </tr> <tr> <td>Bicycle level of service score, BLDS (Eq. 15-31)</td> <td>2.11</td> </tr> <tr> <td>Bicycle level of service (Exhibit 15-4)</td> <td>B</td> </tr> </tbody> </table>				Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h	542.1	Effective width, W_e (Eq. 15-29) ft	29.00	Effective speed factor, S_e (Eq. 15-30)	4.79	Bicycle level of service score, BLDS (Eq. 15-31)	2.11	Bicycle level of service (Exhibit 15-4)	B																				
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h	542.1																																
Effective width, W_e (Eq. 15-29) ft	29.00																																
Effective speed factor, S_e (Eq. 15-30)	4.79																																
Bicycle level of service score, BLDS (Eq. 15-31)	2.11																																
Bicycle level of service (Exhibit 15-4)	B																																
<small>Copyright © 2012 University of Florida, All Rights Reserved HCS 2010™ Version 6.3 Generated: 02/10/2019 16:39</small>																																	

MULTILANE HIGHWAYS WORKSHEET(Direction 1)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N, v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																					
Agency or Company		From/To	SP056 - Pedágio																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 2 - Implantação	Analysis Year	2020																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																						
<input type="checkbox"/> Plan. (vp)																								
Flow Inputs																								
Volume, V (veh/h)	1071	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Level																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	1.2																					
E_T	1.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	50.0																					
Base Free-Flow Speed, BFFS	50.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/ln)	563	Required Number of Lanes, N																						
Speed, S (mi/h)	50.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/ln)	11.3	Max Service Flow Rate (pc/h/ln)																						
LOS	B	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			563.7																					
Effective width, W_v (Eq. 15-29) ft			29.00																					
Effective speed factor, S_f (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.13																					
Bicycle level of service (Exhibit 15-4)			B																					
Copyright © 2012 University of Florida, All Rights Reserved		HCS 2010™ Version 6.3																						
		Generated: 02/10/2019 16:40																						

MULTILANE HIGHWAYS WORKSHEET(Direction 1)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N, v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																					
Agency or Company		From/To	SP056 - Pedágio																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 3 - Operação Futura	Analysis Year	2023																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																						
<input type="checkbox"/> Plan. (v_p)																								
Flow Inputs																								
Volume, V (veh/h)	1128	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Level																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	1.2																					
E_T	1.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	50.0																					
Base Free-Flow Speed, BFFS	50.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/n)	593	Required Number of Lanes, N																						
Speed, S (mi/h)	50.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/n)	11.9	Max Service Flow Rate (pc/h/n)																						
LOS	B	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			593.7																					
Effective width, W_e (Eq. 15-29) ft			29.00																					
Effective speed factor, S_e (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.16																					
Bicycle level of service (Exhibit 15-4)			B																					

MULTILANE HIGHWAYS WORKSHEET(Direction 1)																																			
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D											
Application	Input	Output																																	
Operational (LOS)	FFS, N, v_p	LOS, S, D																																	
Design (N)	FFS, LOS, v_p	N, S, D																																	
Design (v_p)	FFS, LOS, N	v_p , S, D																																	
Planning (LOS)	FFS, N, AADT	LOS, S, D																																	
Planning (N)	FFS, LOS, AADT	N, S, D																																	
Planning (v_p)	FFS, LOS, N	v_p , S, D																																	
<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">General Information</th> <th colspan="2">Site Information</th> </tr> </thead> <tbody> <tr> <td>Analyst</td> <td>TTC Engenharia</td> <td>Highway/Direction to Travel</td> <td>Rod. Cônego Domênico Rangoni</td> </tr> <tr> <td>Agency or Company</td> <td></td> <td>From/To</td> <td>SP056 - Pedágio</td> </tr> <tr> <td>Date Performed</td> <td>02/10/2019</td> <td>Jurisdiction</td> <td></td> </tr> <tr> <td>Analysis Time Period</td> <td>Cenário 4 - Operação Futura</td> <td>Analysis Year</td> <td>2033</td> </tr> </tbody> </table>		General Information		Site Information		Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni	Agency or Company		From/To	SP056 - Pedágio	Date Performed	02/10/2019	Jurisdiction		Analysis Time Period	Cenário 4 - Operação Futura	Analysis Year	2033	<p>Project Description: Estudo de Tráfego - URE Valoriza</p> <p> <input type="checkbox"/> Oper.(LOS) <input type="checkbox"/> Des. (N) <input type="checkbox"/> Plan. (vp) </p>													
General Information		Site Information																																	
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																																
Agency or Company		From/To	SP056 - Pedágio																																
Date Performed	02/10/2019	Jurisdiction																																	
Analysis Time Period	Cenário 4 - Operação Futura	Analysis Year	2033																																
<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="4">Flow Inputs</th> </tr> </thead> <tbody> <tr> <td>Volume, V (veh/h)</td> <td>1417</td> <td>Peak-Hour Factor, PHF</td> <td>0.95</td> </tr> <tr> <td>AADT(veh/h)</td> <td></td> <td>%Trucks and Buses, P_T</td> <td>0</td> </tr> <tr> <td>Peak-Hour Prop of AADT (veh/d)</td> <td></td> <td>%RVs, P_R</td> <td>0</td> </tr> <tr> <td>Peak-Hour Direction Prop, D</td> <td></td> <td>General Terrain:</td> <td>Level</td> </tr> <tr> <td>DDHV (veh/h)</td> <td></td> <td>Grade Length (mi)</td> <td>0.00</td> </tr> <tr> <td>Driver Type Adjustment</td> <td>1.00</td> <td>Up/Down %</td> <td>0.00</td> </tr> <tr> <td></td> <td></td> <td>Number of Lanes</td> <td>2</td> </tr> </tbody> </table>				Flow Inputs				Volume, V (veh/h)	1417	Peak-Hour Factor, PHF	0.95	AADT(veh/h)		%Trucks and Buses, P_T	0	Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0	Peak-Hour Direction Prop, D		General Terrain:	Level	DDHV (veh/h)		Grade Length (mi)	0.00	Driver Type Adjustment	1.00	Up/Down %	0.00			Number of Lanes	2
Flow Inputs																																			
Volume, V (veh/h)	1417	Peak-Hour Factor, PHF	0.95																																
AADT(veh/h)		%Trucks and Buses, P_T	0																																
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																																
Peak-Hour Direction Prop, D		General Terrain:	Level																																
DDHV (veh/h)		Grade Length (mi)	0.00																																
Driver Type Adjustment	1.00	Up/Down %	0.00																																
		Number of Lanes	2																																
<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="4">Calculate Flow Adjustments</th> </tr> </thead> <tbody> <tr> <td>f_p</td> <td>1.00</td> <td>E_R</td> <td>1.2</td> </tr> <tr> <td>E_T</td> <td>1.5</td> <td>f_{HV}</td> <td>1.000</td> </tr> </tbody> </table>				Calculate Flow Adjustments				f_p	1.00	E_R	1.2	E_T	1.5	f_{HV}	1.000																				
Calculate Flow Adjustments																																			
f_p	1.00	E_R	1.2																																
E_T	1.5	f_{HV}	1.000																																
<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Speed Inputs</th> </tr> </thead> <tbody> <tr> <td>Lane Width, LW (ft)</td> <td>12.0</td> </tr> <tr> <td>Total Lateral Clearance, LC (ft)</td> <td>12.0</td> </tr> <tr> <td>Access Points, A (A/mi)</td> <td>0</td> </tr> <tr> <td>Median Type, M</td> <td>Divded</td> </tr> <tr> <td>FFS (measured)</td> <td></td> </tr> <tr> <td>Base Free-Flow Speed, BFFS</td> <td>50.0</td> </tr> </tbody> </table>		Speed Inputs		Lane Width, LW (ft)	12.0	Total Lateral Clearance, LC (ft)	12.0	Access Points, A (A/mi)	0	Median Type, M	Divded	FFS (measured)		Base Free-Flow Speed, BFFS	50.0	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Calc Speed Adj and FFS</th> </tr> </thead> <tbody> <tr> <td>f_{W} (mi/h)</td> <td>0.0</td> </tr> <tr> <td>f_{LC} (mi/h)</td> <td>0.0</td> </tr> <tr> <td>f_A (mi/h)</td> <td>0.0</td> </tr> <tr> <td>f_M (mi/h)</td> <td>0.0</td> </tr> <tr> <td>FFS (mi/h)</td> <td>50.0</td> </tr> </tbody> </table>		Calc Speed Adj and FFS		f_{W} (mi/h)	0.0	f_{LC} (mi/h)	0.0	f_A (mi/h)	0.0	f_M (mi/h)	0.0	FFS (mi/h)	50.0						
Speed Inputs																																			
Lane Width, LW (ft)	12.0																																		
Total Lateral Clearance, LC (ft)	12.0																																		
Access Points, A (A/mi)	0																																		
Median Type, M	Divded																																		
FFS (measured)																																			
Base Free-Flow Speed, BFFS	50.0																																		
Calc Speed Adj and FFS																																			
f_{W} (mi/h)	0.0																																		
f_{LC} (mi/h)	0.0																																		
f_A (mi/h)	0.0																																		
f_M (mi/h)	0.0																																		
FFS (mi/h)	50.0																																		
<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Operations</th> </tr> </thead> <tbody> <tr> <td colspan="2"><u>Operational (LOS)</u></td> </tr> <tr> <td>Flow Rate, v_p (pc/h/lane)</td> <td>745</td> </tr> <tr> <td>Speed, S (mi/h)</td> <td>50.0</td> </tr> <tr> <td>D (pc/mi/h)</td> <td>14.9</td> </tr> <tr> <td>LOS</td> <td>B</td> </tr> </tbody> </table>		Operations		<u>Operational (LOS)</u>		Flow Rate, v_p (pc/h/lane)	745	Speed, S (mi/h)	50.0	D (pc/mi/h)	14.9	LOS	B	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Design</th> </tr> </thead> <tbody> <tr> <td colspan="2"><u>Design (N)</u></td> </tr> <tr> <td>Required Number of Lanes, N</td> <td></td> </tr> <tr> <td>Flow Rate, v_p (pc/h)</td> <td></td> </tr> <tr> <td>Max Service Flow Rate (pc/h/lane)</td> <td></td> </tr> <tr> <td>Design LOS</td> <td></td> </tr> </tbody> </table>		Design		<u>Design (N)</u>		Required Number of Lanes, N		Flow Rate, v_p (pc/h)		Max Service Flow Rate (pc/h/lane)		Design LOS									
Operations																																			
<u>Operational (LOS)</u>																																			
Flow Rate, v_p (pc/h/lane)	745																																		
Speed, S (mi/h)	50.0																																		
D (pc/mi/h)	14.9																																		
LOS	B																																		
Design																																			
<u>Design (N)</u>																																			
Required Number of Lanes, N																																			
Flow Rate, v_p (pc/h)																																			
Max Service Flow Rate (pc/h/lane)																																			
Design LOS																																			
<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">Bicycle Level of Service</th> </tr> </thead> <tbody> <tr> <td>Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h</td> <td style="text-align: right;">745.8</td> </tr> <tr> <td>Effective width, W_v (Eq. 15-29) ft</td> <td style="text-align: right;">29.00</td> </tr> <tr> <td>Effective speed factor, S_f (Eq. 15-30)</td> <td style="text-align: right;">4.79</td> </tr> <tr> <td>Bicycle level of service score, BLDS (Eq. 15-31)</td> <td style="text-align: right;">2.27</td> </tr> <tr> <td>Bicycle level of service (Exhibit 15-4)</td> <td style="text-align: right;">B</td> </tr> </tbody> </table>				Bicycle Level of Service		Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h	745.8	Effective width, W_v (Eq. 15-29) ft	29.00	Effective speed factor, S_f (Eq. 15-30)	4.79	Bicycle level of service score, BLDS (Eq. 15-31)	2.27	Bicycle level of service (Exhibit 15-4)	B																				
Bicycle Level of Service																																			
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h	745.8																																		
Effective width, W_v (Eq. 15-29) ft	29.00																																		
Effective speed factor, S_f (Eq. 15-30)	4.79																																		
Bicycle level of service score, BLDS (Eq. 15-31)	2.27																																		
Bicycle level of service (Exhibit 15-4)	B																																		

MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N, v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Dom ênico Rangoni																					
Agency or Company		From/To	Rio Jurubatuba - Túneis																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 1 - Situação Atual	Analysis Year	2019																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																						
<input type="checkbox"/> Plan. (v_p)																								
Flow Inputs																								
Volume, V (veh/h)	1536	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Rolling																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	2.0																					
E_T	2.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	56.0																					
Base Free-Flow Speed, BFFS	56.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/n)	808	Required Number of Lanes, N																						
Speed, S (mi/h)	55.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/n)	14.7	Max Service Flow Rate (pc/h/n)																						
LOS	B	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			808.4																					
Effective width, W_v (Eq. 15-29) ft			29.00																					
Effective speed factor, S_f (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.31																					
Bicycle level of service (Exhibit 15-4)			B																					

MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N , v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N , v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																					
Agency or Company		From/To	Rio Jurubatuba - Túneis																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 2 - Implantação	Analysis Year	2020																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																						
<input type="checkbox"/> Plan. (v_p)																								
Flow Inputs																								
Volume, V (veh/h)	1596	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Rolling																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	2.0																					
E_T	2.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	56.0																					
Base Free-Flow Speed, BFFS	56.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/lane)	840	Required Number of Lanes, N																						
Speed, S (mi/h)	55.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/h)	15.3	Max Service Flow Rate (pc/h/lane)																						
LOS	B	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			840.0																					
Effective width, W_e (Eq. 15-29) ft			29.00																					
Effective speed factor, S_e (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.33																					
Bicycle level of service (Exhibit 15-4)			B																					

MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N, v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																					
Agency or Company		From/To	Rio Jurubatuba - Túneis																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 3 - Operação Futura	Analysis Year	2023																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS) <input type="checkbox"/> Des. (N) <input type="checkbox"/> Plan. (v_p)																								
Flow Inputs																								
Volume, V (veh/h)	1683	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Rolling																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	2.0																					
E_T	2.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	56.0																					
Base Free-Flow Speed, BFFS	56.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/lane)	885	Required Number of Lanes, N																						
Speed, S (mi/h)	55.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/h)	16.1	Max Service Flow Rate (pc/h/lane)																						
LOS	B	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			885.8																					
Effective width, W_e (Eq. 15-29) ft			29.00																					
Effective speed factor, S_e (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.36																					
Bicycle level of service (Exhibit 15-4)			B																					
Copyright © 2012 University of Florida. All Rights Reserved		HCS 2010™ Version 6.3																						
		Generated: 02/10/2019 16:48																						

MULTILANE HIGHWAYS WORKSHEET(Direction 2)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N, v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																					
Agency or Company		From/To	Rio Jurubatuba - Túneis																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 4 - Operação Futura	Analysis Year	2033																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS) <input type="checkbox"/> Des. (N) <input type="checkbox"/> Plan. (v_p)																								
Flow Inputs																								
Volume, V (veh/h)	2113	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Rolling																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	2.0																					
E_T	2.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	56.0																					
Base Free-Flow Speed, BFFS	56.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/n)	1112	Required Number of Lanes, N																						
Speed, S (mi/h)	55.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/n)	20.2	Max Service Flow Rate (pc/h/n)																						
LOS	C	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			1112.1																					
Effective width, W_v (Eq. 15-29) ft			29.00																					
Effective speed factor, S_f (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.48																					
Bicycle level of service (Exhibit 15-4)			B																					

MULTILANE HIGHWAYS WORKSHEET(Direction 1)																																	
	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>	Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D											
Application	Input	Output																															
Operational (LOS)	FFS, N, v_p	LOS, S, D																															
Design (N)	FFS, LOS, v_p	N, S, D																															
Design (v_p)	FFS, LOS, N	v_p , S, D																															
Planning (LOS)	FFS, N, AADT	LOS, S, D																															
Planning (N)	FFS, LOS, AADT	N, S, D																															
Planning (v_p)	FFS, LOS, N	v_p , S, D																															
<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">General Information</th> <th colspan="2">Site Information</th> </tr> </thead> <tbody> <tr> <td>Analyst</td> <td>TTC Engenharia</td> <td>Highway/Direction to Travel</td> <td>Rod. Cônego Dom ênico Rangoni</td> </tr> <tr> <td>Agency or Company</td> <td></td> <td>From/To</td> <td>Rio Jurubatuba - Túneis</td> </tr> <tr> <td>Date Performed</td> <td>02/10/2019</td> <td>Jurisdiction</td> <td></td> </tr> <tr> <td>Analysis Time Period</td> <td>Cenário 1 - Situação Atual</td> <td>Analysis Year</td> <td>2019</td> </tr> <tr> <td colspan="4">Project Description: Estudo de Tráfego - URE Valoriza</td> </tr> <tr> <td colspan="2" style="text-align: center;"><input type="checkbox"/> Oper.(LOS)</td> <td colspan="2" style="text-align: center;"><input type="checkbox"/> Des. (N)</td> </tr> <tr> <td colspan="4" style="text-align: center;"><input type="checkbox"/> Plan. (v_p)</td> </tr> </tbody> </table>		General Information		Site Information		Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Dom ênico Rangoni	Agency or Company		From/To	Rio Jurubatuba - Túneis	Date Performed	02/10/2019	Jurisdiction		Analysis Time Period	Cenário 1 - Situação Atual	Analysis Year	2019	Project Description: Estudo de Tráfego - URE Valoriza				<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)		<input type="checkbox"/> Plan. (v_p)			
General Information		Site Information																															
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Dom ênico Rangoni																														
Agency or Company		From/To	Rio Jurubatuba - Túneis																														
Date Performed	02/10/2019	Jurisdiction																															
Analysis Time Period	Cenário 1 - Situação Atual	Analysis Year	2019																														
Project Description: Estudo de Tráfego - URE Valoriza																																	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																															
<input type="checkbox"/> Plan. (v_p)																																	
Flow Inputs																																	
Volume, V (veh/h)	1429	Peak-Hour Factor, PHF	0.95																														
AADT(veh/h)		%Trucks and Buses, P_T	0																														
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																														
Peak-Hour Direction Prop, D		General Terrain:	Rolling																														
DDHV (veh/h)		Grade Length (mi)	0.00																														
Driver Type Adjustment	1.00	Up/Down %	0.00																														
		Number of Lanes	2																														
Calculate Flow Adjustments																																	
f_p	1.00	E_R	2.0																														
E_T	2.5	f_{HV}	1.000																														
Speed Inputs		Calc Speed Adj and FFS																															
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																														
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																														
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																														
Median Type, M	Divded	f_M (mi/h)	0.0																														
FFS (measured)		FFS (mi/h)	56.0																														
Base Free-Flow Speed, BFFS	56.0																																
Operations		Design																															
<u>Operational (LOS)</u> Flow Rate, v_p (pc/h/n) 752 Speed, S (mi/h) 55.0 D (pc/mi/n) 13.7 LOS B		<u>Design (N)</u> Required Number of Lanes, N Flow Rate, v_p (pc/h) Max Service Flow Rate (pc/h/n) Design LOS																															
Bicycle Level of Service																																	
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			752.1																														
Effective width, W_e (Eq. 15-29) ft			29.00																														
Effective speed factor, S_e (Eq. 15-30)			4.79																														
Bicycle level of service score, BLDS (Eq. 15-31)			2.28																														
Bicycle level of service (Exhibit 15-4)			B																														

MULTILANE HIGHWAYS WORKSHEET(Direction 1)																																	
	<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>	Application	Input	Output	Operational (LOS)	FFS, N , v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D											
Application	Input	Output																															
Operational (LOS)	FFS, N , v_p	LOS, S, D																															
Design (N)	FFS, LOS, v_p	N, S, D																															
Design (v_p)	FFS, LOS, N	v_p , S, D																															
Planning (LOS)	FFS, N, AADT	LOS, S, D																															
Planning (N)	FFS, LOS, AADT	N, S, D																															
Planning (v_p)	FFS, LOS, N	v_p , S, D																															
<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">General Information</th> <th colspan="2">Site Information</th> </tr> </thead> <tbody> <tr> <td>Analyst</td> <td>TTC Engenharia</td> <td>Highway/Direction to Travel</td> <td>Rod. Cônego Domênico Rangoni</td> </tr> <tr> <td>Agency or Company</td> <td></td> <td>From/To</td> <td>Rio Jurubatuba - Túneis</td> </tr> <tr> <td>Date Performed</td> <td>02/10/2019</td> <td>Jurisdiction</td> <td></td> </tr> <tr> <td>Analysis Time Period</td> <td>Cenário 2 - Implantação</td> <td>Analysis Year</td> <td>2020</td> </tr> <tr> <td colspan="4">Project Description: Estudo de Tráfego - URE Valoriza</td> </tr> <tr> <td colspan="2" style="text-align: center;"><input type="checkbox"/> Oper.(LOS)</td> <td colspan="2" style="text-align: center;"><input type="checkbox"/> Des. (N)</td> </tr> <tr> <td colspan="4" style="text-align: center;"><input type="checkbox"/> Plan. (v_p)</td> </tr> </tbody> </table>		General Information		Site Information		Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni	Agency or Company		From/To	Rio Jurubatuba - Túneis	Date Performed	02/10/2019	Jurisdiction		Analysis Time Period	Cenário 2 - Implantação	Analysis Year	2020	Project Description: Estudo de Tráfego - URE Valoriza				<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)		<input type="checkbox"/> Plan. (v_p)			
General Information		Site Information																															
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																														
Agency or Company		From/To	Rio Jurubatuba - Túneis																														
Date Performed	02/10/2019	Jurisdiction																															
Analysis Time Period	Cenário 2 - Implantação	Analysis Year	2020																														
Project Description: Estudo de Tráfego - URE Valoriza																																	
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																															
<input type="checkbox"/> Plan. (v_p)																																	
Flow Inputs <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td>Volume, V (veh/h)</td> <td>1486</td> <td>Peak-Hour Factor, PHF</td> <td>0.95</td> </tr> <tr> <td>AADT (veh/h)</td> <td></td> <td>%Trucks and Buses, P_T</td> <td>0</td> </tr> <tr> <td>Peak-Hour Prop of AADT (veh/d)</td> <td></td> <td>%RVs, P_R</td> <td>0</td> </tr> <tr> <td>Peak-Hour Direction Prop, D</td> <td></td> <td>General Terrain:</td> <td>Rolling</td> </tr> <tr> <td>DDHV (veh/h)</td> <td></td> <td>Grade Length (mi)</td> <td>0.00</td> </tr> <tr> <td>Driver Type Adjustment</td> <td>1.00</td> <td>Up/Down %</td> <td>0.00</td> </tr> <tr> <td></td> <td></td> <td>Number of Lanes</td> <td>2</td> </tr> </tbody> </table>				Volume, V (veh/h)	1486	Peak-Hour Factor, PHF	0.95	AADT (veh/h)		%Trucks and Buses, P_T	0	Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0	Peak-Hour Direction Prop, D		General Terrain:	Rolling	DDHV (veh/h)		Grade Length (mi)	0.00	Driver Type Adjustment	1.00	Up/Down %	0.00			Number of Lanes	2		
Volume, V (veh/h)	1486	Peak-Hour Factor, PHF	0.95																														
AADT (veh/h)		%Trucks and Buses, P_T	0																														
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																														
Peak-Hour Direction Prop, D		General Terrain:	Rolling																														
DDHV (veh/h)		Grade Length (mi)	0.00																														
Driver Type Adjustment	1.00	Up/Down %	0.00																														
		Number of Lanes	2																														
Calculate Flow Adjustments <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td>f_p</td> <td>1.00</td> <td>E_R</td> <td>2.0</td> </tr> <tr> <td>E_T</td> <td>2.5</td> <td>f_{HV}</td> <td>1.000</td> </tr> </tbody> </table>				f_p	1.00	E_R	2.0	E_T	2.5	f_{HV}	1.000																						
f_p	1.00	E_R	2.0																														
E_T	2.5	f_{HV}	1.000																														
Speed Inputs <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td>Lane Width, LW (ft)</td> <td>12.0</td> </tr> <tr> <td>Total Lateral Clearance, LC (ft)</td> <td>12.0</td> </tr> <tr> <td>Access Points, A (A/mi)</td> <td>0</td> </tr> <tr> <td>Median Type, M</td> <td>Divded</td> </tr> <tr> <td>FFS (measured)</td> <td></td> </tr> <tr> <td>Base Free-Flow Speed, BFFS</td> <td>56.0</td> </tr> </tbody> </table>		Lane Width, LW (ft)	12.0	Total Lateral Clearance, LC (ft)	12.0	Access Points, A (A/mi)	0	Median Type, M	Divded	FFS (measured)		Base Free-Flow Speed, BFFS	56.0	Calc Speed Adj and FFS <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td>f_{W} (mi/h)</td> <td>0.0</td> </tr> <tr> <td>f_{LC} (mi/h)</td> <td>0.0</td> </tr> <tr> <td>f_A (mi/h)</td> <td>0.0</td> </tr> <tr> <td>f_M (mi/h)</td> <td>0.0</td> </tr> <tr> <td>FFS (mi/h)</td> <td>56.0</td> </tr> </tbody> </table>		f_{W} (mi/h)	0.0	f_{LC} (mi/h)	0.0	f_A (mi/h)	0.0	f_M (mi/h)	0.0	FFS (mi/h)	56.0								
Lane Width, LW (ft)	12.0																																
Total Lateral Clearance, LC (ft)	12.0																																
Access Points, A (A/mi)	0																																
Median Type, M	Divded																																
FFS (measured)																																	
Base Free-Flow Speed, BFFS	56.0																																
f_{W} (mi/h)	0.0																																
f_{LC} (mi/h)	0.0																																
f_A (mi/h)	0.0																																
f_M (mi/h)	0.0																																
FFS (mi/h)	56.0																																
Operations <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td colspan="2"><u>Operational (LOS)</u></td> </tr> <tr> <td>Flow Rate, v_p (pc/h/ln)</td> <td>782</td> </tr> <tr> <td>Speed, S (mi/h)</td> <td>55.0</td> </tr> <tr> <td>D (pc/mi/ln)</td> <td>14.2</td> </tr> <tr> <td>LOS</td> <td>B</td> </tr> </tbody> </table>		<u>Operational (LOS)</u>		Flow Rate, v_p (pc/h/ln)	782	Speed, S (mi/h)	55.0	D (pc/mi/ln)	14.2	LOS	B	Design <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td colspan="2"><u>Design (N)</u></td> </tr> <tr> <td>Required Number of Lanes, N</td> <td></td> </tr> <tr> <td>Flow Rate, v_p (pc/h)</td> <td></td> </tr> <tr> <td>Max Service Flow Rate (pc/h/ln)</td> <td></td> </tr> <tr> <td>Design LOS</td> <td></td> </tr> </tbody> </table>		<u>Design (N)</u>		Required Number of Lanes, N		Flow Rate, v_p (pc/h)		Max Service Flow Rate (pc/h/ln)		Design LOS											
<u>Operational (LOS)</u>																																	
Flow Rate, v_p (pc/h/ln)	782																																
Speed, S (mi/h)	55.0																																
D (pc/mi/ln)	14.2																																
LOS	B																																
<u>Design (N)</u>																																	
Required Number of Lanes, N																																	
Flow Rate, v_p (pc/h)																																	
Max Service Flow Rate (pc/h/ln)																																	
Design LOS																																	
Bicycle Level of Service <table border="1" style="width:100%; border-collapse: collapse;"> <tbody> <tr> <td>Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h</td> <td>782.1</td> </tr> <tr> <td>Effective width, W_e (Eq. 15-29) ft</td> <td>29.00</td> </tr> <tr> <td>Effective speed factor, S_e (Eq. 15-30)</td> <td>4.79</td> </tr> <tr> <td>Bicycle level of service score, BLDS (Eq. 15-31)</td> <td>2.30</td> </tr> <tr> <td>Bicycle level of service (Exhibit 15-4)</td> <td>B</td> </tr> </tbody> </table>				Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h	782.1	Effective width, W_e (Eq. 15-29) ft	29.00	Effective speed factor, S_e (Eq. 15-30)	4.79	Bicycle level of service score, BLDS (Eq. 15-31)	2.30	Bicycle level of service (Exhibit 15-4)	B																				
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h	782.1																																
Effective width, W_e (Eq. 15-29) ft	29.00																																
Effective speed factor, S_e (Eq. 15-30)	4.79																																
Bicycle level of service score, BLDS (Eq. 15-31)	2.30																																
Bicycle level of service (Exhibit 15-4)	B																																

MULTILANE HIGHWAYS WORKSHEET(Direction 1)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N, v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N, v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																					
Agency or Company		From/To	Rio Jurubatuba - Túneis																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 3 - Operação Futura	Analysis Year	2023																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS) <input type="checkbox"/> Des. (N) <input type="checkbox"/> Plan. (v_p)																								
Flow Inputs																								
Volume, V (veh/h)	1565	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Rolling																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	2.0																					
E_T	2.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	56.0																					
Base Free-Flow Speed, BFFS	56.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/n)	823	Required Number of Lanes, N																						
Speed, S (mi/h)	55.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/n)	15.0	Max Service Flow Rate (pc/h/n)																						
LOS	B	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			823.7																					
Effective width, W_e (Eq. 15-29) ft			29.00																					
Effective speed factor, S_e (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.32																					
Bicycle level of service (Exhibit 15-4)			B																					

MULTILANE HIGHWAYS WORKSHEET(Direction 1)																								
		<table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Application</th> <th>Input</th> <th>Output</th> </tr> </thead> <tbody> <tr> <td>Operational (LOS)</td> <td>FFS, N, v_p</td> <td>LOS, S, D</td> </tr> <tr> <td>Design (N)</td> <td>FFS, LOS, v_p</td> <td>N, S, D</td> </tr> <tr> <td>Design (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> <tr> <td>Planning (LOS)</td> <td>FFS, N, AADT</td> <td>LOS, S, D</td> </tr> <tr> <td>Planning (N)</td> <td>FFS, LOS, AADT</td> <td>N, S, D</td> </tr> <tr> <td>Planning (v_p)</td> <td>FFS, LOS, N</td> <td>v_p, S, D</td> </tr> </tbody> </table>		Application	Input	Output	Operational (LOS)	FFS, N , v_p	LOS, S, D	Design (N)	FFS, LOS, v_p	N, S, D	Design (v_p)	FFS, LOS, N	v_p , S, D	Planning (LOS)	FFS, N, AADT	LOS, S, D	Planning (N)	FFS, LOS, AADT	N, S, D	Planning (v_p)	FFS, LOS, N	v_p , S, D
Application	Input	Output																						
Operational (LOS)	FFS, N , v_p	LOS, S, D																						
Design (N)	FFS, LOS, v_p	N, S, D																						
Design (v_p)	FFS, LOS, N	v_p , S, D																						
Planning (LOS)	FFS, N, AADT	LOS, S, D																						
Planning (N)	FFS, LOS, AADT	N, S, D																						
Planning (v_p)	FFS, LOS, N	v_p , S, D																						
General Information		Site Information																						
Analyst	TTC Engenharia	Highway/Direction to Travel	Rod. Cônego Domênico Rangoni																					
Agency or Company		From/To	Rio Jurubatuba - Túneis																					
Date Performed	02/10/2019	Jurisdiction																						
Analysis Time Period	Cenário 4 - Operação Futura	Analysis Year	2033																					
Project Description: Estudo de Tráfego - URE Valoriza																								
<input type="checkbox"/> Oper.(LOS)		<input type="checkbox"/> Des. (N)																						
<input type="checkbox"/> Plan. (v_p)																								
Flow Inputs																								
Volume, V (veh/h)	1965	Peak-Hour Factor, PHF	0.95																					
AADT(veh/h)		%Trucks and Buses, P_T	0																					
Peak-Hour Prop of AADT (veh/d)		%RVs, P_R	0																					
Peak-Hour Direction Prop, D		General Terrain:	Rolling																					
DDHV (veh/h)		Grade Length (mi)	0.00																					
Driver Type Adjustment	1.00	Up/Down %	0.00																					
		Number of Lanes	2																					
Calculate Flow Adjustments																								
f_p	1.00	E_R	2.0																					
E_T	2.5	f_{HV}	1.000																					
Speed Inputs		Calc Speed Adj and FFS																						
Lane Width, LW (ft)	12.0	f_{W} (mi/h)	0.0																					
Total Lateral Clearance, LC (ft)	12.0	f_{LC} (mi/h)	0.0																					
Access Points, A (A/mi)	0	f_A (mi/h)	0.0																					
Median Type, M	Divded	f_M (mi/h)	0.0																					
FFS (measured)		FFS (mi/h)	56.0																					
Base Free-Flow Speed, BFFS	56.0																							
Operations		Design																						
<u>Operational (LOS)</u>		<u>Design (N)</u>																						
Flow Rate, v_p (pc/h/lane)	1034	Required Number of Lanes, N																						
Speed, S (mi/h)	55.0	Flow Rate, v_p (pc/h)																						
D (pc/mi/h)	18.8	Max Service Flow Rate (pc/h/lane)																						
LOS	C	Design LOS																						
Bicycle Level of Service																								
Directional demand flow rate in outside lane, v_{OL} (Eq. 15-24) veh/h			1034.2																					
Effective width, W_e (Eq. 15-29) ft			29.00																					
Effective speed factor, S_e (Eq. 15-30)			4.79																					
Bicycle level of service score, BLDS (Eq. 15-31)			2.44																					
Bicycle level of service (Exhibit 15-4)			B																					
Copyright © 2012 University of Florida, All Rights Reserved		HCS 2010™ Version 6.3																						
		Generated: 02/10/2019 16:50																						

ANEXO 03

Relatórios Sidra Intersection 8.0

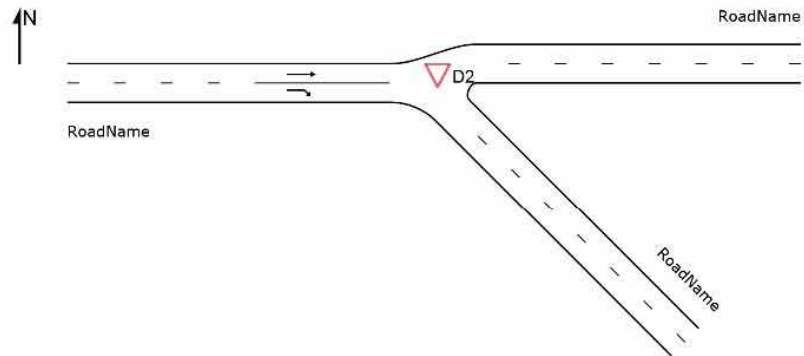
SITE LAYOUT

▽ Site: D2 [Divergência 2 - Cenário1]

Divergência 2

Site Category: (None)

Giveaway / Yield (Two-Way)



SIDRA INTERSECTION8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Created: quarta-feira, 2 de outubro de 2019 17:59:33

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

LANE LEVEL OF SERVICE

Lane Level of Service

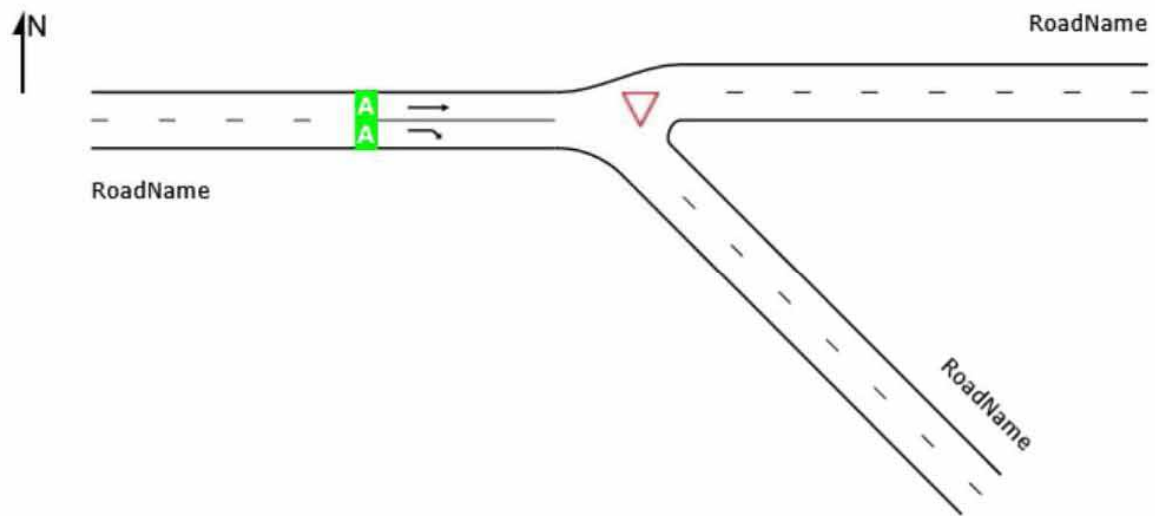
▽ Site: D2 [Divergência 2 - Cenário1]

Divergência 2

Site Category: (None)

Giveaway / Yield (Two-Way)

LOS	Approaches	Intersection
	West	
LOS	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:50

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

▽ Site: D2 [Divergência 2 - Cenário1]

Divergência 2
Site Category: (None)
Giveaway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
West: RoadName												
11	T1	88	10,7	0,048	0,0	LOS A	0,0	0,0	0,00	0,00	0,00	40,0
12a	R1	200	90,5	0,156	4,4	LOS A	0,0	0,0	0,00	0,52	0,00	36,2
Approach		288	66,1	0,156	3,1	NA	0,0	0,0	0,00	0,36	0,00	37,4
All Vehicles		288	66,1	0,156	3,1	NA	0,0	0,0	0,00	0,36	0,00	37,4

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:50

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

LANE LEVEL OF SERVICE

Lane Level of Service

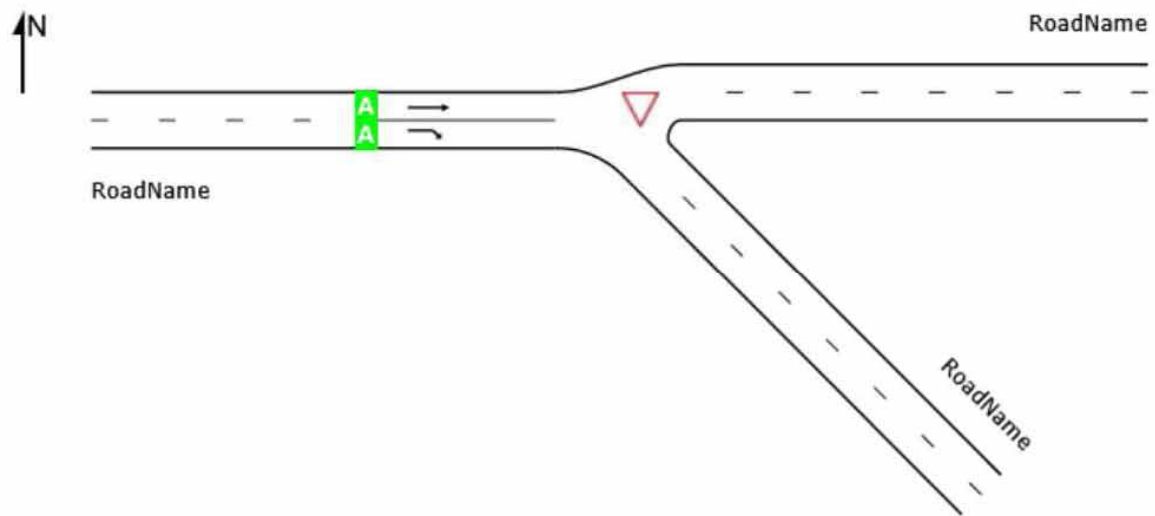
▽ Site: D2 [Divergência 2 - Cenário2]

Divergência 2

Site Category: (None)

Giveaway / Yield (Two-Way)

LOS	Approaches	Intersection
	West	
LOS	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:50

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

▽ Site: D2 [Divergência 2 - Cenário2]

Divergência 2
Site Category: (None)
Giveaway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
West: RoadName												
11	T1	91	10,5	0,049	0,0	LOS A	0,0	0,0	0,00	0,00	0,00	40,0
12a	R1	208	90,4	0,163	4,4	LOS A	0,0	0,0	0,00	0,52	0,00	36,2
Approach		299	66,2	0,163	3,1	NA	0,0	0,0	0,00	0,36	0,00	37,4
All Vehicles		299	66,2	0,163	3,1	NA	0,0	0,0	0,00	0,36	0,00	37,4

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:50

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

LANE LEVEL OF SERVICE

Lane Level of Service

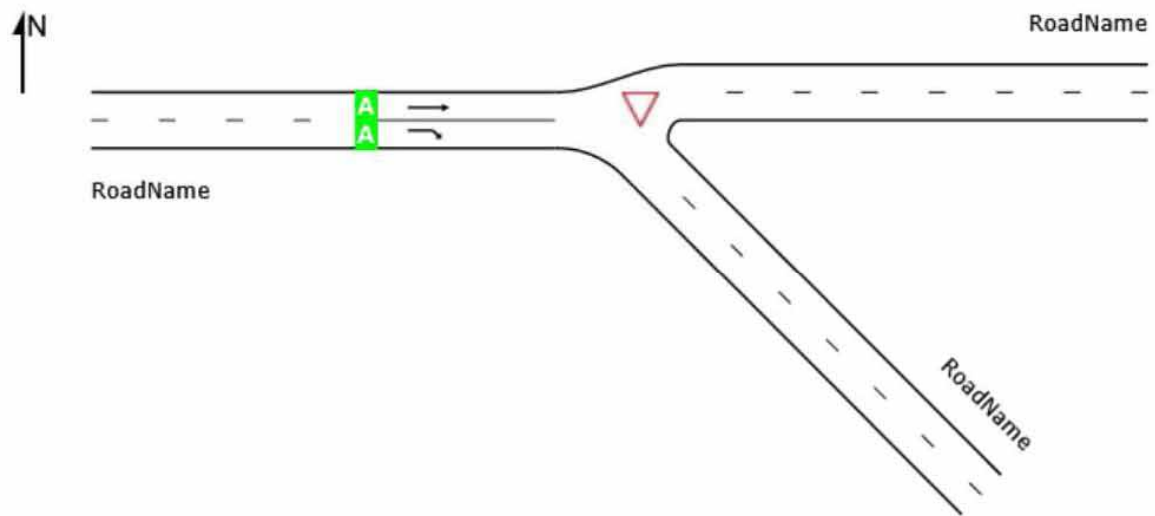
▽ Site: D2 [Divergência 2 - Cenário3]

Divergência 2

Site Category: (None)

Giveaway / Yield (Two-Way)

LOS	Approaches	Intersection
	West	
LOS	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:51

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

▽ Site: D2 [Divergência 2 - Cenário3]

Divergência 2
Site Category: (None)
Giveaway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
West: RoadName												
11	T1	97	10,9	0,052	0,0	LOS A	0,0	0,0	0,00	0,00	0,00	40,0
12a	R1	219	90,4	0,171	4,5	LOS A	0,0	0,0	0,00	0,52	0,00	36,2
Approach		316	66,0	0,171	3,1	NA	0,0	0,0	0,00	0,36	0,00	37,4
All Vehicles		316	66,0	0,171	3,1	NA	0,0	0,0	0,00	0,36	0,00	37,4

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:51

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

LANE LEVEL OF SERVICE

Lane Level of Service

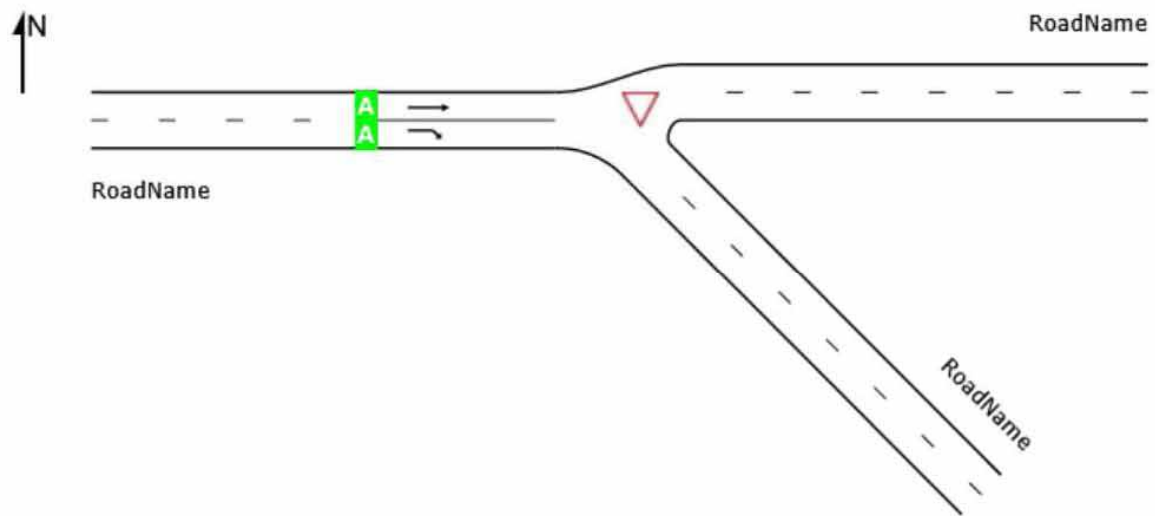
▽ Site: D2 [Divergência 2 - Cenário4]

Divergência 2

Site Category: (None)

Giveaway / Yield (Two-Way)

LOS	Approaches	Intersection
	West	
LOS	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:51

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

▽ Site: D2 [Divergência 2 - Cenário4]

Divergência 2
Site Category: (None)
Giveaway / Yield (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
West: RoadName												
11	T1	122	10,3	0,066	0,0	LOS A	0,0	0,0	0,00	0,00	0,00	40,0
12a	R1	275	90,8	0,215	4,5	LOS A	0,0	0,0	0,00	0,52	0,00	36,2
Approach		397	66,0	0,215	3,1	NA	0,0	0,0	0,00	0,36	0,00	37,4
All Vehicles		397	66,0	0,215	3,1	NA	0,0	0,0	0,00	0,36	0,00	37,4

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:51

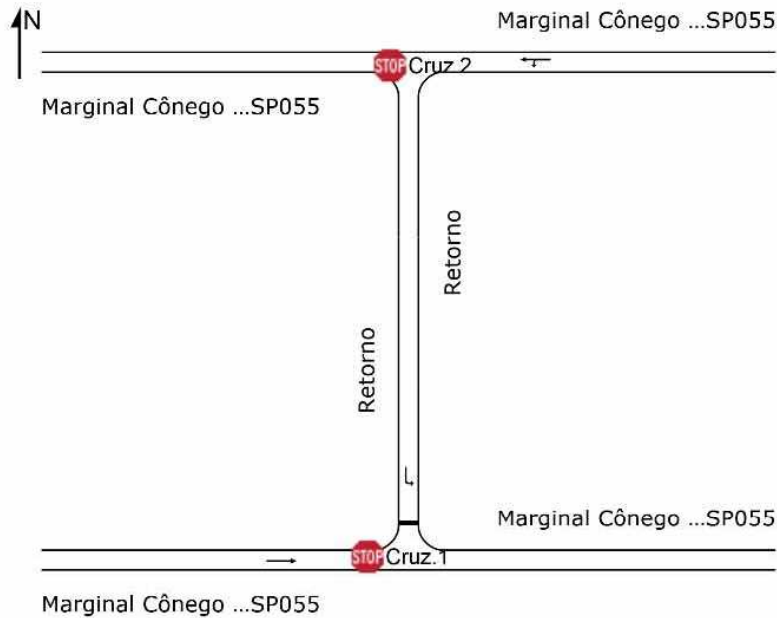
Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

NETWORK LAYOUT

Network: N101 [Cruzamentos - Cenário 1]

New Network

Network Category: (None)



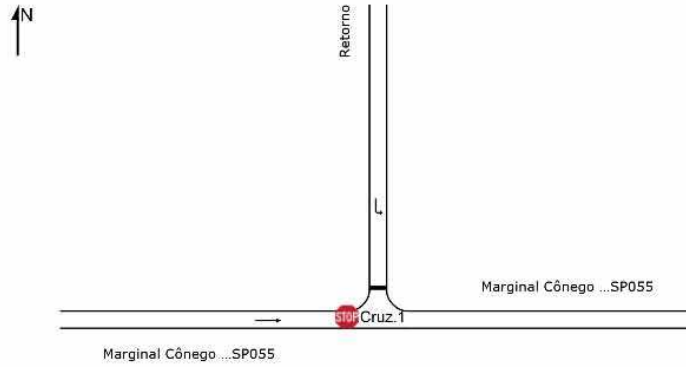
SITES IN NETWORK		
Site ID	CCG ID	Site Name
STOPCruz.1	NA	Cruzamento 1 - Cenário 1
STOPCruz.2	NA	Cruzamento 2 - Cenário 1

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com
 Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Created: quarta-feira, 2 de outubro de 2019 18:14:00
 Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

SITE LAYOUT

 Site: Cruz.1 [Cruzamento 1 - Cenário 1]

New Site
Site Category: (None)
Stop (Two-Way)



SIDRA INTERSECTION8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com
 Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Created: quarta-feira, 2 de outubro de 2019 18:05:20
 Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

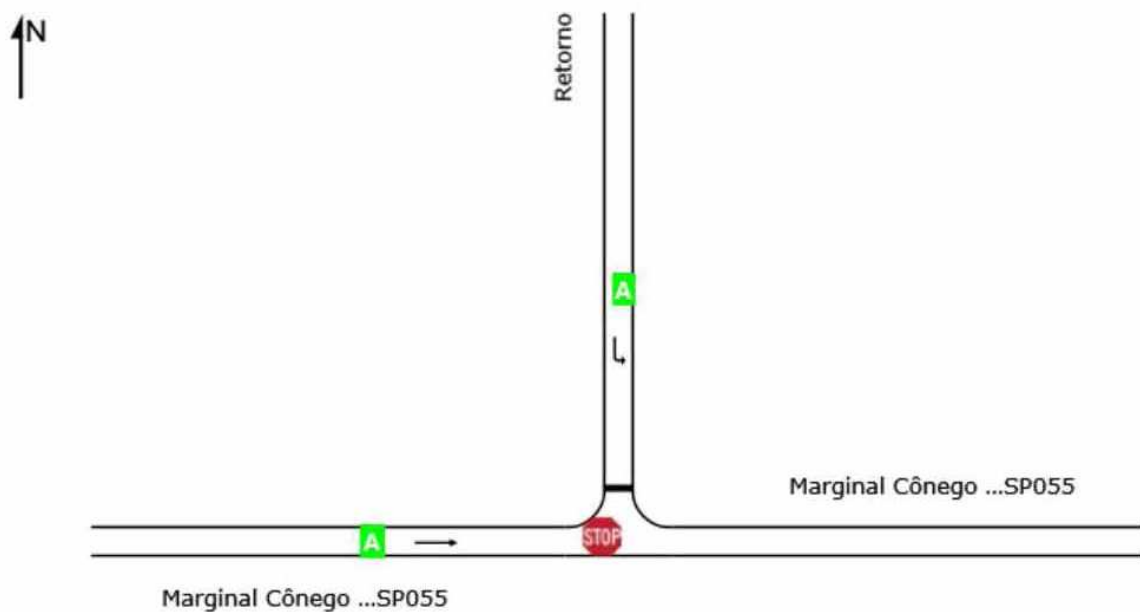
LANE LEVEL OF SERVICE

Lane Level of Service

STOP Site: Cruz1 [Cruzamento 1 - Cenário 1]

New Site
Site Category: (None)
Stop (Two-Way)

	Approaches		Intersection
	North	West	
LOS	A	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:47

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

 Site: Cruz.1 [Cruzamento 1 - Cenário 1]

New Site
Site Category: (None)
Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
North: Retomo												
7	L2	59	73,2	0,068	8,7	LOS A	0,2	2,6	0,18	0,96	0,18	34,7
Approach		59	73,2	0,068	8,7	LOS A	0,2	2,6	0,18	0,96	0,18	34,7
West: Marginal Cônego Domênico Rangoni - SP055												
11	T1	60	50,9	0,039	0,0	LOS A	0,0	0,0	0,00	0,00	0,00	40,0
Approach		60	50,9	0,039	0,0	NA	0,0	0,0	0,00	0,00	0,00	40,0
All Vehicles		119	61,9	0,068	4,3	NA	0,2	2,6	0,09	0,47	0,09	37,7

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:47

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

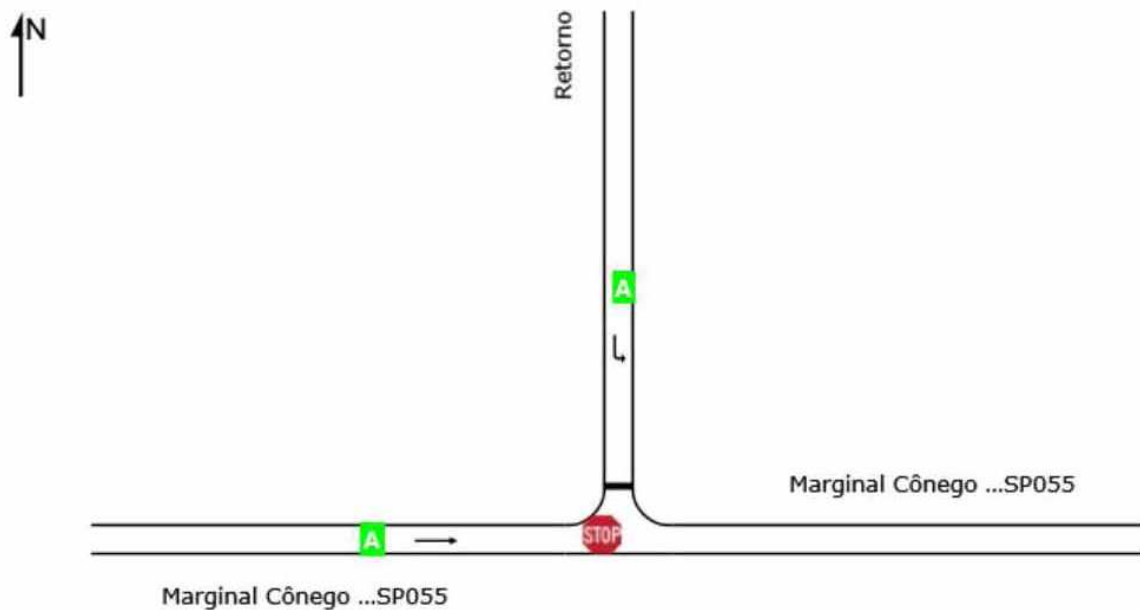
LANE LEVEL OF SERVICE

Lane Level of Service

STOP Site: Cruz1 [Cruzamento 1 - Cenário 2]

New Site
Site Category: (None)
Stop (Two-Way)

	Approaches		Intersection
	North	West	
LOS	A	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if v/c > 1 irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:47

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

 Site: Cruz.1 [Cruzamento 1 - Cenário 2]

New Site
Site Category: (None)
Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
North: Retomo												
7	L2	74	71,4	0,085	8,7	LOS A	0,3	3,2	0,19	0,95	0,19	34,7
Approach		74	71,4	0,085	8,7	LOS A	0,3	3,2	0,19	0,95	0,19	34,7
West: Marginal Cônego Domênico Rangoni - SP055												
11	T1	62	50,8	0,040	0,0	LOS A	0,0	0,0	0,00	0,00	0,00	40,0
Approach		62	50,8	0,040	0,0	NA	0,0	0,0	0,00	0,00	0,00	40,0
All Vehicles		136	62,0	0,085	4,7	NA	0,3	3,2	0,10	0,52	0,10	37,5

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:47

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

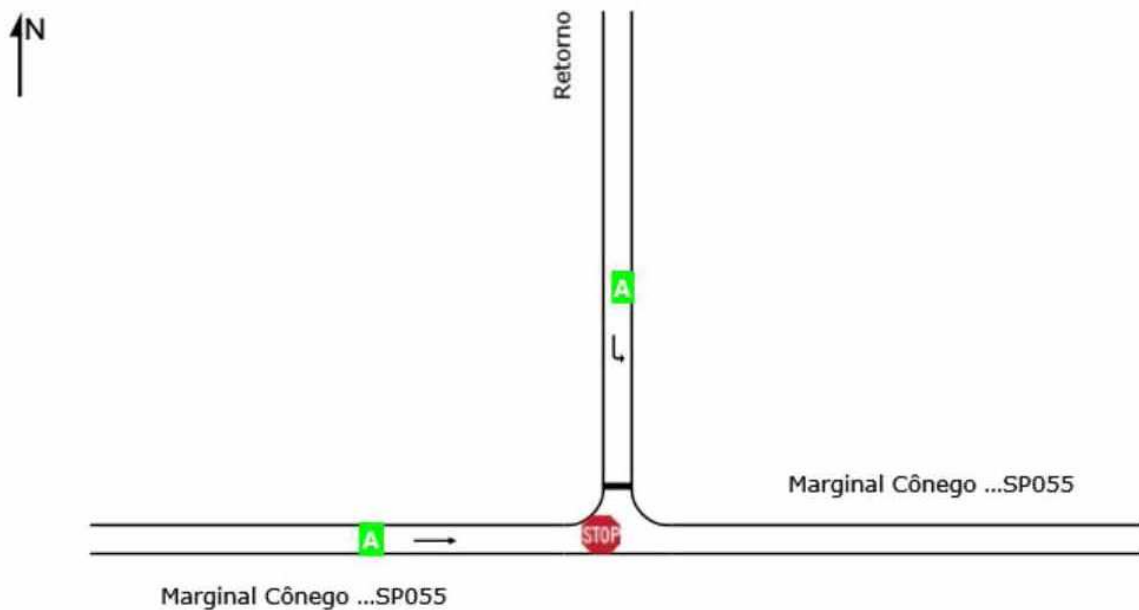
LANE LEVEL OF SERVICE

Lane Level of Service

STOP Site: Cruz 1 [Cruzamento 1 - Cenário 3]

New Site
Site Category: (None)
Stop (Two-Way)

	Approaches		Intersection
	North	West	
LOS	A	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:47

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

 Site: Cruz.1 [Cruzamento 1 - Cenário 3]

New Site
Site Category: (None)
Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
North: Retomo												
7	L2	64	73,8	0,075	8,8	LOS A	0,2	2,8	0,19	0,95	0,19	34,6
Approach		64	73,8	0,075	8,8	LOS A	0,2	2,8	0,19	0,95	0,19	34,6
West: Marginal Cônego Domênico Rangoni - SP055												
11	T1	66	50,8	0,043	0,0	LOS A	0,0	0,0	0,00	0,00	0,00	40,0
Approach		66	50,8	0,043	0,0	NA	0,0	0,0	0,00	0,00	0,00	40,0
All Vehicles		131	62,1	0,075	4,3	NA	0,2	2,8	0,09	0,47	0,09	37,7

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:47

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

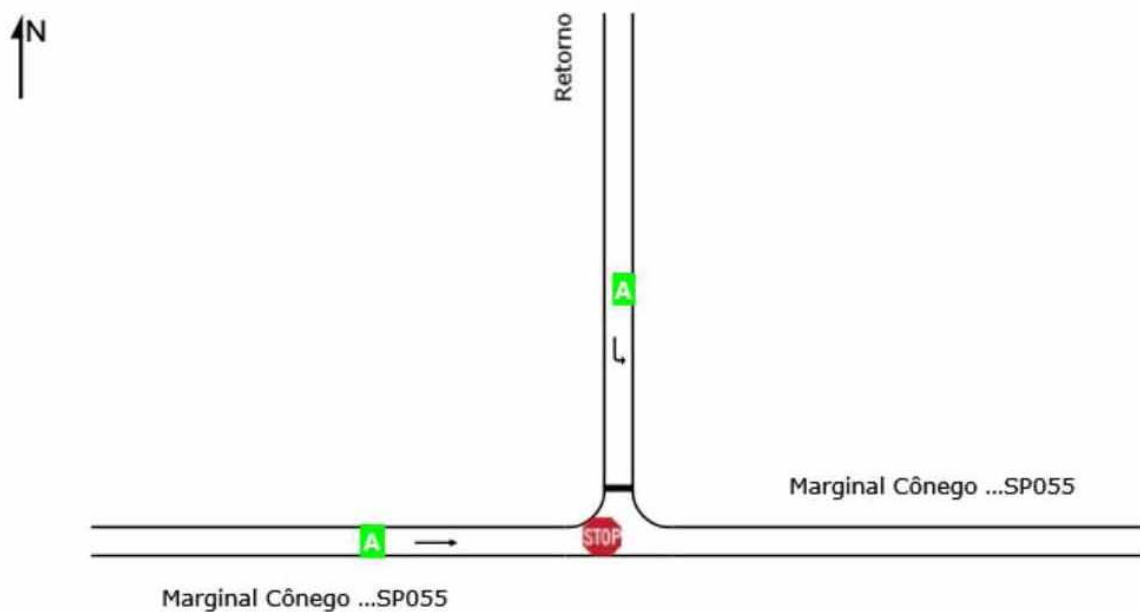
LANE LEVEL OF SERVICE

Lane Level of Service

STOP Site: Cruz1 [Cruzamento 1 - Cenário 4]

New Site
Site Category: (None)
Stop (Two-Way)

	Approaches		Intersection
	North	West	
LOS	A	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:48

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

 Site: Cruz.1 [Cruzamento 1 - Cenário 4]

New Site
Site Category: (None)
Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
North: Retomo												
7	L2	81	72,7	0,097	8,9	LOS A	0,3	3,7	0,22	0,95	0,22	34,6
Approach		81	72,7	0,097	8,9	LOS A	0,3	3,7	0,22	0,95	0,22	34,6
West: Marginal Cônego Domênico Rangoni - SP055												
11	T1	83	50,6	0,053	0,0	LOS A	0,0	0,0	0,00	0,00	0,00	40,0
Approach		83	50,6	0,053	0,0	NA	0,0	0,0	0,00	0,00	0,00	40,0
All Vehicles		164	61,5	0,097	4,4	NA	0,3	3,7	0,11	0,47	0,11	37,7

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

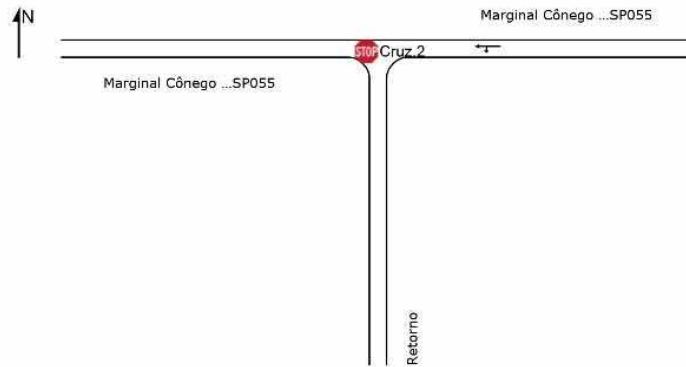
Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:48

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

SITE LAYOUT

 Site: Cruz.2 [Cruzamento 2 - Cenário 1]

New Site
Site Category: (None)
Stop (Two-Way)



SIDRA INTERSECTION8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Created: quarta-feira, 2 de outubro de 2019 18:08:50
Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

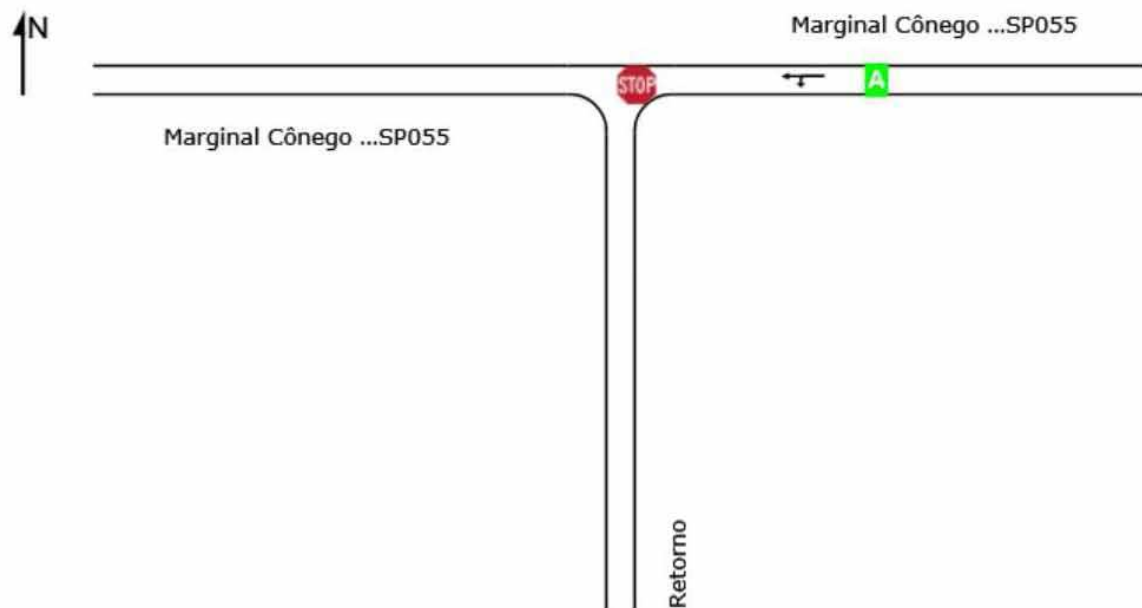
LANE LEVEL OF SERVICE

Lane Level of Service

STOP Site: Cruz.2 [Cruzamento 2 - Cenário 1]

New Site
Site Category: (None)
Stop (Two-Way)

	Approaches East	Intersection
LOS	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRÁFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:48

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

 Site: Cruz.2 [Cruzamento 2 - Cenário 1]

New Site
Site Category: (None)
Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Marginal C6nego Dom6nico Rangoni - SP055												
4	L2	59	73,2	0,199	4,4	LOS A	0,0	0,0	0,00	0,10	0,00	33,7
5	T1	255	36,4	0,199	0,0	LOS A	0,0	0,0	0,00	0,10	0,00	39,7
Approach		314	43,3	0,199	0,8	NA	0,0	0,0	0,00	0,10	0,00	39,0
All Vehicles		314	43,3	0,199	0,8	NA	0,0	0,0	0,00	0,10	0,00	39,0

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:48

Project: H:\PGTC_30 - URE Cubat6o\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

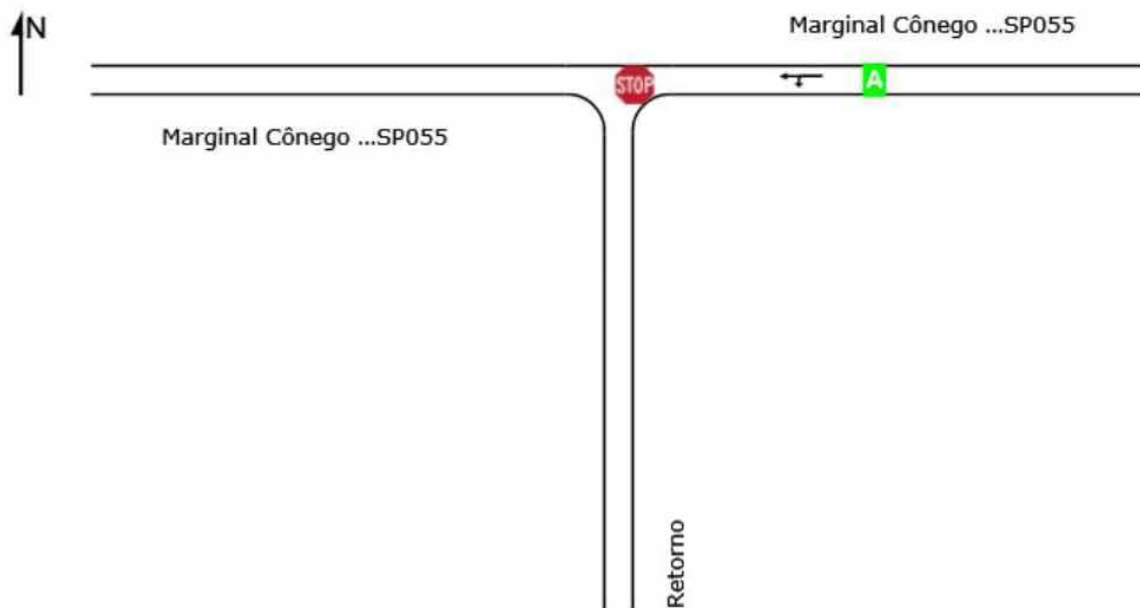
LANE LEVEL OF SERVICE

Lane Level of Service

STOP Site: Cruz.2 [Cruzamento 2 - Cenário 2]

New Site
Site Category: (None)
Stop (Two-Way)

	Approaches East	Intersection
LOS	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRÁFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:49

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

 Site: Cruz.2 [Cruzamento 2 - Cenário 2]

New Site
Site Category: (None)
Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Marginal C6nego Dom6nico Rangoni - SP055												
4	L2	74	71,4	0,214	4,4	LOS A	0,0	0,0	0,00	0,12	0,00	33,6
5	T1	261	36,3	0,214	0,0	LOS A	0,0	0,0	0,00	0,12	0,00	39,6
Approach		335	44,0	0,214	1,0	NA	0,0	0,0	0,00	0,12	0,00	38,8
All Vehicles		335	44,0	0,214	1,0	NA	0,0	0,0	0,00	0,12	0,00	38,8

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:49

Project: H:\PGTC_30 - URE Cubat6o\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

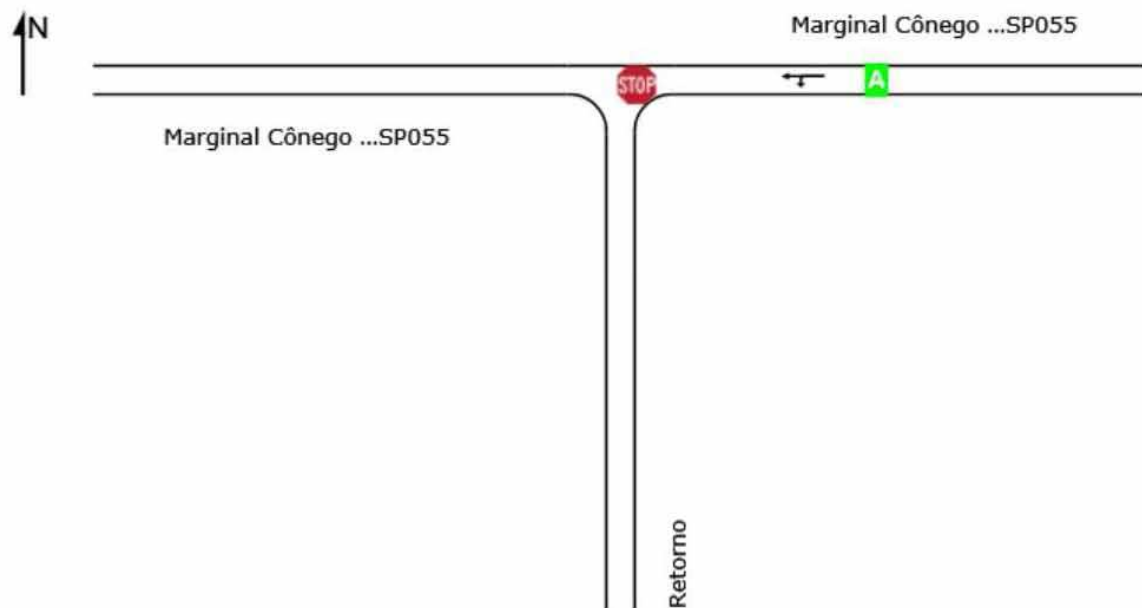
LANE LEVEL OF SERVICE

Lane Level of Service

 Site: Cruz.2 [Cruzamento 2 - Cenário 3]

New Site
Site Category: (None)
Stop (Two-Way)

	Approaches East	Intersection
LOS	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRÁFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:49

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

 Site: Cruz.2 [Cruzamento 2 - Cenário 3]

New Site
Site Category: (None)
Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Marginal Cónego Domênico Rangoni - SP055												
4	L2	64	73,8	0,218	4,4	LOS A	0,0	0,0	0,00	0,10	0,00	33,6
5	T1	279	36,2	0,218	0,0	LOS A	0,0	0,0	0,00	0,10	0,00	39,7
Approach		343	43,3	0,218	0,8	NA	0,0	0,0	0,00	0,10	0,00	39,0
All Vehicles		343	43,3	0,218	0,8	NA	0,0	0,0	0,00	0,10	0,00	39,0

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:49

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

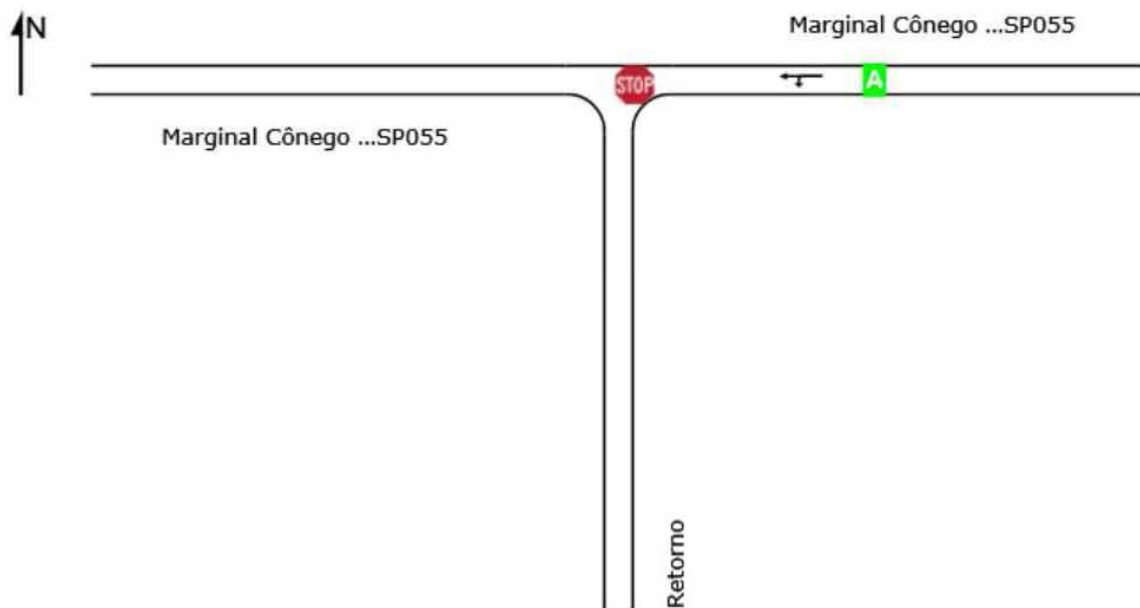
LANE LEVEL OF SERVICE

Lane Level of Service

STOP Site: Cruz.2 [Cruzamento 2 - Cenário 4]

New Site
Site Category: (None)
Stop (Two-Way)

	Approaches East	Intersection
LOS	NA	NA



Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Lane LOS values are based on average delay and v/c ratio (degree of saturation) per lane.

LOS F will result if $v/c > 1$ irrespective of lane delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all lanes (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road lanes.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRÁFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:50

Project: H:\PGTC_30 - URE Cubatão\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

MOVEMENT SUMMARY

 Site: Cruz.2 [Cruzamento 2 - Cenário 4]

New Site
Site Category: (None)
Stop (Two-Way)

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: Marginal C6nego Dom6nico Rangoni - SP055												
4	L2	81	72,7	0,274	4,4	LOS A	0,0	0,0	0,00	0,10	0,00	33,7
5	T1	351	36,3	0,274	0,0	LOS A	0,0	0,0	0,00	0,10	0,00	39,7
Approach		432	43,2	0,274	0,8	NA	0,0	0,0	0,00	0,10	0,00	39,0
All Vehicles		432	43,2	0,274	0,8	NA	0,0	0,0	0,00	0,10	0,00	39,0

Site Level of Service (LOS) Method: Delay & v/c (HCM 2010). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay and v/c ratio (degree of saturation) per movement.

LOS F will result if v/c > 1 irrespective of movement delay value (does not apply for approaches and intersection).

Minor Road Approach LOS values are based on average delay for all movements (v/c not used as specified in HCM 2010).

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2018 Akcelik and Associates Pty Ltd | sidrasolutions.com

Organisation: TTC ENGENHARIA DE TRAFEGO E TRANSPORTES | Processed: quarta-feira, 2 de outubro de 2019 17:58:50

Project: H:\PGTC_30 - URE Cubat6o\TEC\Sidra\PGTC30_Cruzamentos_rev1.sip8

ÍNDICE

APRESENTAÇÃO	3
1. CARACTERIZAÇÃO.....	4
1.1. Localização	4
1.2. Características do Empreendimento.....	5
1.3. Operação Atual	6
1.3.1. Transporte de Resíduos	6
1.3.2. Número de Funcionários na Operação Atual	7
1.4. Fase de Implantação	7
1.4.1. Transporte de Equipamentos e Materiais.....	7
1.4.2. Número de Funcionários na Fase de Implantação.....	12
1.5. Operação Futura	21
1.5.1. Coleta e Transporte de Resíduos	21
1.5.2. Número de Funcionários na Fase de Operação do Empreendimento.....	25
2. FLUXO DE PASSAGEM.....	28
2.1. Fluxo de Passagem Existente	28
2.2. Fatores Aplicados aos Dados de Fluxo	33
3. COMPOSIÇÃO DOS CENÁRIOS DE ANÁLISE	42
4. ESTIMATIVA DE DEMANDA.....	43
4.1. Geração de Viagens.....	43
4.2. Divisão Modal.....	44
4.3. Distribuição Temporal.....	45
5. ANÁLISE DO IMPACTO	46
5.1. Segmentos Analisados.....	47
5.2. Resultados	49
6. CONCLUSÃO	50
6.1. Avaliação Final.....	50
7. ANEXOS	51
ÍNDICE.....	175